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www.koverthawkins.com

ADDENDUM No. 1

Project: **Fire Station No. 1 Project**
Town of Clarksville
Clarksville, IN
Proj. No: **1639.02**

Date: February 15, 2018

This addendum is a part of the bid documents. Acknowledge receipt on the Proposal Form.

General

1. Notify Matt Gullo of any questions regarding bidding at phone (812-913-4616) or email (matt.gullo@koverthawkins.com).
2. Pre-bid meeting set for Tuesday February 20, 2018 at Clarksville Town Hall 2nd Floor Conference Rm.

Specification

None

Drawings

Structural Drawings – Additional Drawings

- Add the following drawings to the Construction Document Set
  - S-001 (General Structural Notes)
  - S-002 (General Structural Details)
  - S-101 (Foundation Plan)
  - S-102 (Roof Framing Plan)
  - S-201 (Foundation Sections and Details)
  - S-202 (Foundation Section and Details)
  - S-301 (Framing Section and Details)
  - S-302 (Framing Section and Details)
  - S-303 (Framing Section and Details)
  - S-304 (Hose Tower Details)
  - S-305 (Framing and Section Details)
  - S-306 (Framing and Section Details)

Electrical Drawing E-302

- Add additional E-302 drawing to the Construction Document Set for the "Electrical Riser Diagram".

Prepared by,

Matthew D. Gullo, RLA
Director of Landscape Architecture and Planning

enclosed: Electrical and Structural Drawings

file: 1639.02

End of Addendum No. 1

Registered Architect: Indiana #LA21300004; KY#835; CLARB#33114
Direct: 812. 913.4616
Email: matt.gullo@koverthawkins.com

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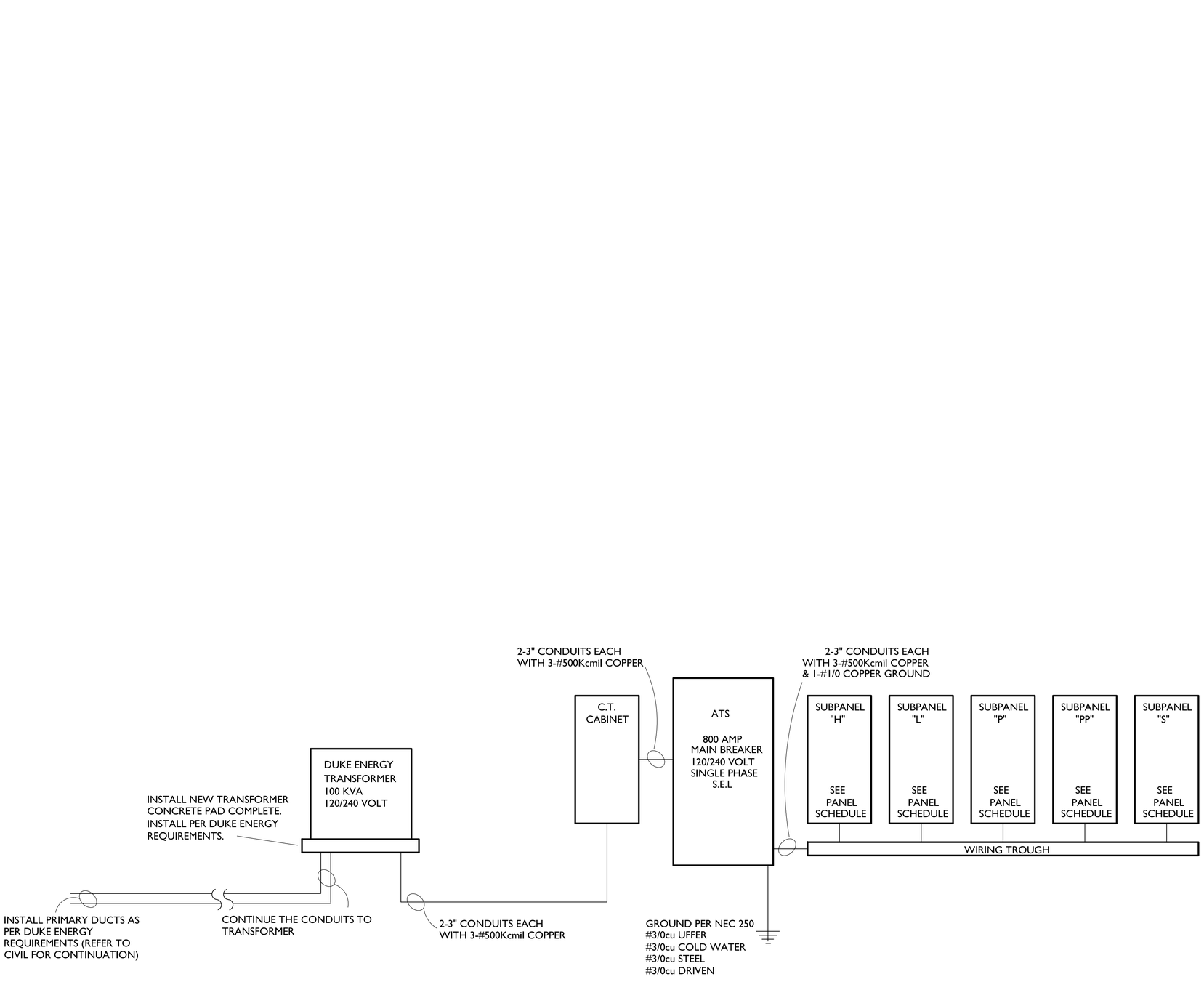
KovertHawkins architects

Revisions 1
2
3
4
5
6

Drawn BB
Checked By BB
Project No. 1639.02
Date 11/28/2017

BRENDAN T. BROWN REGISTERED ARCHITECT STATE OF INDIANA AR11000070

Certified By



ELECTRIC RISER DIAGRAM

N.T.S.

Notice

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Clarksville Fire Department Station No. 1

106 E. Stansifer Avenue
Clarksville, IN 47129

Town of Clarksville

New Facility

Sheet

E-302

STRUCTURAL GENERAL NOTES

STRUCTURAL DESIGN CRITERIA

1. APPLICABLE BUILDING CODES:
   A. 2014 INDIANA BUILDING CODE
   B. 2012 INTERNATIONAL BUILDING CODE
   C. ASCE STANDARD: ASCE 7–10
2. PROJECT LOCATION: CLARKSVILLE, INDIANA (CLARK COUNTY)
3. DESIGN LOADS:
   A. FLOOR LIVE LOADS
      1. SLABS ON GRADE ....100 PSF
      2. APPARATUS BAY FLOOR .... 250 PSF
   B. ROOF LOAD .... 20 PSF
   C. SNOW LOADS
      1. GROUND SNOW LOAD: P_g = 20 PSF
      2. FLAT–ROOF SNOW LOAD: P_f = 16.8 PSF
      3. SNOW EXPOSURE FACTOR: C_e = 1.0
      4. THERMAL FACTOR: C_t = 1.0
      5. SNOW LOAD IMPORTANCE FACTOR: I_S = 1.2
   D. WIND LOADS
      1. BASIC WIND SPEED = 120 MPH (3–SECOND GUSTS – ULTIMATE WIND)
      2. WIND IMPORTANCE FACTOR: I_W = 1.0
      3. EXPOSURE CATEGORY: B
      4. DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING:
         P = 22.68 PSF (INTERIOR ZONE) & 35.07 PSF (EDGE STRIP)
      5. DESIGN WIND PRESSURE FOR MAIN WINDFORCE RESISTING SYSTEM: P = 22.10 PSF (ULTIMATE)
   E. EARTHQUAKE LOADS
      1. OCCUPANCY CATEGORY: IV (ESSENTIAL FACILITY)
      2. MAPPED SPECTRAL RESPONSE ACCELERATION: S_S=20.6%, S_1=10.6%
      3. SEISMIC DESIGN CATEGORY C
      4. SITE CLASS C
      5. RESPONSE MODIFCATION FACTOR = 3.5
      6. SYSTEM OVERSTRENGTH FACTOR = 2.5
      7. DEFLECTION AMPLIFICATION FACTOR = 2.25
      8. SEISMIC IMPORTANCE FACTOR = 1.50
      9. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
      10. SPECTRAL RESPONSE COEFFICIENTS S_{DS} = 0.164, S_{D1}= 0.120
      11. C_S = 0.047
      12. DESIGN BASE SHEAR = 0.047 W
      13. BASIC STRUCTURAL SYSTEM: INTERMEDIATE REINFORCED MASONRY SHEAR WALLS

FOUNDATIONS

1. ASSUMED ALLOWABLE SOIL BEARING PRESSURE:
   A. CONTINUOUS WALL FOOTINGS .... 3,000 PSF
   B. ISOLATED COLUMN FOOTINGS .... 3,000 PSF
2. FOUNDATIONS HAVE BEEN SIZED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEERING STUDY (SOILS REPORT) PREPARED BY ASHER ENGINEERING, INC., DATED NOVEMBER 27, 2017 (ASHER PROJECT NO. 17–119).
3. THE OWNER SHALL RETAIN A QUALIFIED GEOTECHNICAL ENGINEER TO INSPECT AND APPROVE ALL BEARING SURFACES AND EARTHWORK OPERATIONS.
4. THE CONTRACTOR SHALL THOROUGHLY FAMILIARIZE HIMSELF WITH THE FINDINGS AND RECOMMENDATIONS OF THE SOILS REPORT AND SHALL PERFORM ALL EARTHWORK OPERATIONS AND FOUNDATION INSTALLATION OPERATIONS IN ACCORDANCE WITH THESE RECOMMENDATIONS AND THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER.
5. PRIOR TO CONSTRUCTION OF ANY PERMANENT STRUCTURE, ALL EXISTING SURFACE FILL, ALL TOPSOIL AND ORGANIC MATERIAL, ALL WET, SOFT, LOOSE, OR UNDESIRABLE SOIL, AND ALL OLD ABANDONED CONCRETE SHALL BE REMOVED TO THE EXTENT RECOMMENDED BY THE SOILS REPORT AND THE GEOTECHNICAL ENGINEER.
6. CONCRETE FOR FOOTINGS SHALL BE PLACED THE SAME DAY EXCAVATIONS ARE OPENED. IF THIS IS IMPOSSIBLE, STEPS SHALL BE TAKEN TO ADEQUATELY PROTECT THE OPEN EXCAVATION.
7. FOOTINGS SHALL BEAR ON COMPETENT, SUITABLE STIFF UNDISTURBED ORIGINAL SOIL OR PROPERLY COMPACTED CONTROLLED ENGINEERED FILL, AS RECOMMENDED BY THE SOILS REPORT AND THE GEOTECHNICAL ENGINEER.
8. SLABS ON GRADE SHALL BEAR ON A 6–INCH THICK LAYER OF FREE DRAINING GRANULAR MATERIAL AS SPECIFIED IN THE SOILS REPORT. FOLLOW THE PROCEDURES RECOMMENDED IN THE SOILS REPORT AND THE INSTRUCTIONS OF THE GEOTECHNICAL ENGINEER.
9. ENGINEERED FILL & BACKFILL SHALL BE PLACED AND COMPACTED ACCORDING TO THE RECOMMENDATIONS OF THE SOILS REPORT, AND THE GEOTECHNICAL ENGINEER.
10. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER BEFORE CONCRETE IS PLACED. THE ADEQUACY OF THE SOIL BEARING SURFACE SHALL BE DETERMINED BY THE GEOTECHNICAL ENGINEER. ALL GEOTECHNICAL FIELD REPORTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER.
11. PROVIDE CONTROL AND CONSTRUCTION JOINTS IN SLAB ON GRADE AS SPECIFIED BY THE ARCHITECT (MAXIMUM SPACING = 20 FEET +/– EACH DIRECTION). CONTRACTOR SHALL SUBMIT THE SLAB JOINT PLAN TO THE ARCHITECT FOR APPROVAL PRIOR TO PLACING CONCRETE FOR THE FLOOR SLAB (SEE CONCRETE GENERAL NOTE NO. 17 FOR CONTROL JOINT INSTALLATION).

CONCRETE

1. ALL CONCRETE FOR GENERAL USE (INCLUDING FOOTINGS AND FOUNDATION WALLS) SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI.
2. REINFORCING STEEL SHALL BE AS FOLLOWS:
   STIRRUPS AND TIES .... ASTM A615 GRADE 60
   ALL OTHER REINFORCING .... ASTM A615 GRADE 60
   WELDED WIRE FABRIC* .... ASTM A185
3. PROVIDE BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH ACI DETAILING MANUAL. ALL BAR SUPPORTS, IN AREAS WHERE CONCRETE WILL BE EXPOSED, SHALL HAVE PLASTIC FEET. PRECAST CONCRETE (fc'=3000psi) BLOCKS 3"x3"x3" SHALL BE USED TO SUPPORT REINFORCING OFF OF THE GROUND. AT ALL OTHER LOCATIONS, CHAIRS OR STANDEES SHALL BE USED.
4. DETAILING, FABRICATION AND PLACING OF REINFORCING SHALL CONFORM TO APPLICABLE PROVISIONS OF ACI 315 AND ACI 318.
5. SLABS, FOUNDATION WALLS, AND FOOTINGS SHALL HAVE NO HORIZONTAL JOINTS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL KEYED BULKHEADS. ALL REINFORCEMENT SHALL CONTINUE THROUGH JOINTS.
6. BEFORE PLACING CONCRETE, THE CONTRACTOR SHALL NOTIFY ALL SUBCONTRACTORS TO BE SURE ALL SLEEVES, CONDUIT, CHASES, ETC. ARE PROPERLY INSTALLED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER AS SOON AS PRACTICAL, BUT AT LEAST 24 HOURS PRIOR TO PLACING CONCRETE TO ALLOW FOR INSPECTION OF REINFORCING AND EMBEDDED ITEMS.

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CONCRETE CONTINUED

7. MATERIALS SHALL COMPLY WITH REQUIREMENTS OF DESIGNATED SPECIFICATIONS OF AMERICAN SOCIETY FOR TESTING AND MATERIALS, 1916 RACE STREET, PHILADELPHIA, PENNSYLVANIA.
8. CONSTRUCTION PROCEDURES SHALL COMPLY WITH RECOMMENDATIONS SET FORTH IN DESIGNATED STANDARDS OF AMERICAN CONCRETE INSTITUTE, P.O. BOX 9094, FARMINGTON HILLS, MICHIGAN 48333.
9. ADMIXTURE OTHER THAN AIR–ENTRAINING SHALL NOT BE USED WITHOUT APPROVAL OF THE ARCHITECT/ENGINEER. AIR–ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C260.
10. CURING COMPOUND SHALL CONFORM TO FEDERAL SPECIFICATION TT–C800A, AND A.S.T.M. C309. THE MATERIAL SHALL BE EQUAL TO SONNEBORN KUR–N–SEAL, MASTERSEAL, BY MASTER BUILDERS, OR CLEAR SEAL, BY W.R. GRACE.
11. ALL REINFORCING SPLICES SHALL BE CLASS B TENSION LAP SPLICE.
12. SPREAD BARS AROUND SMALL OPENINGS AND SLEEVES IN SLABS AND WALLS WHERE POSSIBLE AND WHERE BAR SPACING WILL NOT EXCEED 1.5 TIMES THE NORMAL SPACING. DISCONTINUE BARS AT LARGE OPENINGS WHERE NECESSARY AND PROVIDE AN AREA OF REINFORCEMENT EQUAL TO THE INTERRUPTED REINFORCEMENT, DISTRIBUTING ONE–HALF OF THIS REINFORCEMENT EACH SIDE OF THE OPENING (CLASS B TENSION LAP SPLICED). HOLES LARGER THAN 12 INCHES IN ANY DIRECTION SHALL HAVE (1) #5x5'–0" DIAGONAL BAR IN BOTH FACES AT EACH CORNER.
13. PIER REINFORCEMENT SHALL BE DOWELED TO THE FOOTING. PROVIDE DOWELS EQUAL IN SIZE, NUMBER AND GRADE TO THE PIER REINFORCEMENT UNLESS OTHERWISE INDICATED. DOWELS SHALL BE HOOKED 90 DEGREES AT THE BOTTOM LEVEL OF FOOTING REINFORCEMENT. DOWELS SHALL BE LAPPED WITH THE PIER REINFORCEMENT.
14. ALL VERTICAL CONCRETE SURFACES SHALL BE FORMED. HOWEVER, VERTICAL SURFACES OF FOOTINGS MAY BE EARTH–FORMED IF THE SOIL IS SUFFICIENTLY STIFF TO PREVENT CAVE–INS.
15. REINFORCING BARS SHALL BE IN PLACE AND SECURED PRIOR TO POURING CONCRETE. "STICKING" OF REINFORCING AFTER CONCRETE IS PLACED IS PROHIBITED.
16. REINFORCING BAR SHOP DRAWINGS SHALL SHOW NUMBER, SIZE AND LOCATION OF BARS, AS WELL AS LAP LENGTH AND CLEAR COVER.
17. UNLESS NOTED ON THE PLANS, ALL CONCRETE SLABS SUPPORTED BY SOIL OR GRANULAR SUB–BASE SHALL CONTAIN CONTROL JOINTS AND CONSTRUCTION JOINTS AS SPECIFIED BY THE ARCHITECT, AT SPACINGS NOT TO EXCEED 20 FEET ON CENTER IN BOTH DIRECTIONS. SAW–CUT JOINTS SHALL BE INSTALLED AS SOON AS THE CONCRETE IS HARD ENOUGH TO WITHSTAND SAWING WITHOUT RAVELLING JOINT EDGES OR DISLODGING COARSE AGGREGATE PARTICLES. LIGHTWEIGHT EARLY–CUT SAWS SHALL BE USED. CONTRACTOR SHALL SUBMIT CONSTRUCTION AND CONTROL JOINT LAYOUT TO THE ARCHITECT FOR APPROVAL PROIR TO PLACING CONCRETE SLABS.
18. DOVETAIL SLOTS SHALL BE INSTALLED IN ALL CONCRETE WORK WHICH IS TO RECEIVE BRICK VENEER OR OTHER FACING MATERIALS.

REINFORCED MASONRY

1. COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE F'_M = 1500 PSI.
2. GROUT FOR BOND BEAMS AND GROUTED CELLS IN CONCRETE MASONRY UNITS SHALL BE PEA GRAVEL CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI.
3. MORTAR FOR CONCRETE MASONRY SHALL BE TYPE S.
4. PROVIDE CONTINUOUS HORIZONTAL JOINT REINFORCING IN ALL REINFORCED MASONRY WALLS AT 16 INCHES ON CENTER UNLESS NOTED OTHERWISE.
5. SPLICES IN VERTICAL REINFORCEMENT SHALL BE LAPPED A MINIMUM OF 48 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
6. ALL MASONRY WALLS SHALL BE LATERALLY BRACED BY THE CONTRACTOR UNTIL ALL STRUCTURAL FRAMING AND DECKING HAVE BEEN INSTALLED IN UNITS OF CONSTRUCTION ADJACENT TO THE WALLS.
7. UNLESS NOTED OTHERWISE, A BOND BEAM WITH (2) #5 BARS SHALL BE PROVIDED AT THE TOP OF ALL WALLS, AT ALL BEARING ELEVATIONS, AND AT LOCATIONS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS.
8. ALL CMU UNITS LOCATED BELOW GRADE SHALL BE GROUTED SOLID.
9. UNLESS OTHERWISE SHOWN OR NOTED, PLACE (1) #5 FULL–HEIGHT VERTICAL REINFORCING BAR AT ALL WALL CORNERS, ENDS OF WALLS,EACH SIDE OF CONTROL JOINTS, SIDES OF OPENINGS, AND WALL INTERSECTIONS. (UNLESS OTHERWISE SHOWN OR NOTED, PLACE (2) #5 BARS AT SIDES OF OPENINGS 10 FEET WIDE AND GREATER)
4. SECURE ALL VERTICAL REINFORCING STEEL IN CMU WALLS WITH DUR–O–WALL REBAR POSITIONERS OR APPROVED EQUAL.
5. CONTRACTOR SHALL SUBMIT DRAWINGS COORDINATED WITH THE MASONRY AND MEP CONTRACTORS, SHOWING THE MEP PENETRATIONS THROUGH LOAD BEARING WALLS. THESE DRAWINGS SHALL SHOW THE SIZE AND LOCATION OF ALL PENETRATIONS AND SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.
6. PROVIDE VERTICAL CONTROL JOINTS IN ALL REINFORCED MASONRY. COORDINATE LOCATIONS OF CONTROL JOINTS WITH THE ARCHITECT.

STRUCTURAL STEEL

1. ALL ROLLED STEEL PLATES, SHAPES (EXCLUDING WIDE FLANGE SHAPES), BARS AND MISCELLANEOUS ITEMS SHALL BE STRUCTURAL QUALITY CARBON STEEL COMPLYING WITH ASTM A36 (MINIMUM YIELD 36,000 PSI). WIDE FLANGE SHAPES SHALL BE STRUCTURAL QUALITY CARBON STEEL COMPLYING WITH ASTM A992 (MINIMUM YIELD 50,000 PSI).
2. HOLLOW STRUCTURAL SECTIONS (HSS) SHALL COMPLY WITH ASTM A500, GRADE B (MINIMUM YIELD 46 KSI FOR SQUARE AND RECTANGULAR SECTIONS AND 42 KSI FOR ROUND SECTIONS).
3. ALL BOLTED CONNECTIONS SHALL BE MADE WITH 3/4" DIAMETER ASTM F1852, TYPE 1 TWIST–OFF–TYPE TENSION–CONTROL BOLTS IN BEARING–TYPE CONNECTIONS.
4. ANCHOR RODS SHALL COMPLY WITH ASTM F1554, GRADE 36.
5. EXPANSION ANCHORS SHALL BE HILTI CARBON STEEL KWIK BOLT 3 ANCHOR MANUFACTURED BY HILTI FASTENING SYSTEMS, OR APPROVED EQUAL.
6. ADHESIVE ANCHORS SHALL CONSIST OF HAS–E STEEL ANCHOR RODS WITH THE HIT HY200 ADHESIVE SUPPLIED BY HILTI FASTENING SYSTEMS, OR APPROVED EQUAL. INSTALL IN ACCORDANCE WITH THE SUPPLIER'S RECOMMENDATIONS.
7. WELDED HEADED STUDS TO BE USED AS CONCRETE ANCHORS OR SHEAR STUDS SHALL BE LOW CARBON STEEL SOLID FLUXED STUDS COMPLYING WITH ASTM A–108, WITH A MINIMUM Fu=60KSI. STUDS SHALL BE AUTOMATICALLY END WELDED. THE SPECIFIED LENGTH IS THE AFTER WELD LENGTH (AWL).
8. DEFORMED BAR ANCHORS (DBA) SHALL BE LOW CARBON STEEL PER ASTM A496 (Fu=80KSI), AND SHALL BE AUTOMATICALLY END WELDED.
9. ALL WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED TO PERFORM EACH TYPE OF WELD REQUIRED. ALL WELDS AND WELDING PROCEDURES SHALL COMPLY WITH AWS D1.1, USING E70XX ELECTRODES UNLESS NOTED OTHERWISE. ALL WELDS SHALL BE INSPECTED.
10. WELD SIZES NOT SHOWN ON DESIGN DRAWINGS SHALL BE MINIMUM SIZE REQUIRED BY AWS D1.1 (LATEST EDITION) ACCORDING TO THE MATERIAL THICKNESS BEING WELDED. ALL WELDS SHALL BE PRE–QUALIFIED PER AWS D1.1 (LATEST EDITION).
11. STEEL FRAMEWORK SHALL NOT BE ASSUMED STRUCTURALLY STABLE UNTIL ALL MEMBERS ARE IN PLACE AND CONNECTIONS ARE INSTALLED. ANY USE OF THE PARTIALLY ERECTED FRAMEWORK FOR TEMPORARY SUPPORT OF ANY KIND SHALL BE DONE ONLY AT THE CONTRACTOR'S RISK.
12. COMPLY WITH THE PROVISIONS OF THE LATEST EDITIONS OF THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS, EXCEPT AS OTHERWISE SHOWN OR SPECIFIED HEREIN.
    A. A.I.S.C. "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
    B. A.I.S.C. "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."
    C. A.I.S.C. "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
    D. AWS "STRUCTURAL WELDING CODE."

STRUCTURAL STEEL CONTINUED

13. ALL CONNECTIONS NOT INDICATED ON THE DESIGN DRAWINGS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE STRUCTURAL STEEL IS TO BE ERECTED, RETAINED BY THE STEEL FABRICATOR. ALL CALCULATIONS AND SHOP DRAWINGS SHALL BE DULY STAMPED AND SIGNED BY THE REGISTERED STRUCTURAL ENGINEER AND SUBMITTED TO THE ARCHITECT/STRUCTURAL ENGINEER FOR REVIEW. STAMPING AND SIGNING OF SHOP DRAWINGS SHALL BE FOR THE EXCLUSIVE PURPOSE OF CERTIFYING THAT THE CONNECTIONS ARE DETAILED AS PER THE DESIGN PERFORMED BY THE REGISTERED STRUCTURAL ENGINEER. FAILURE TO SUBMIT STAMPED AND SIGNED CALCULATIONS AND STAMPED AND SIGNED SHOP DRAWINGS SHALL BE SUFFICIENT CAUSE FOR REJECTION OF SHOP DRAWINGS. THE CONTRACTOR SHALL BE LIABLE FOR THE DIMENSION, FIT, TOLERANCES, FABRICATION AND ERECTION.
14. THE CONTRACTOR SHALL BE LIABLE FOR DIMENSIONS, FIT, TOLERENCES, FABRICATION, AND ERECTION OF ALL STRUCTURAL STEEL ELEMENTS.
15. SIMPLE SPAN CONNECTIONS FOR BEAMS SHALL CONSIST OF STANDARD DOUBLE–ANGLE BOLTED AND/OR WELDED CONNECTIONS, AND SHALL BE DESIGNED FOR ONE–HALF THE BEAM LOAD CAPACITY AS GIVEN IN AISC TABLE 3–6 "MAXIMUM TOTAL UNIFORM LOAD" (AISC MANUAL, 13TH EDITION).
16. LENGTH OF CONNECTION ANGLES FOR BEAM–TO–COLUMN OR BEAM–TO–BEAM CONNECTIONS SHALL BE THE LARGEST STANDARD LENGTH LESS THAN OR EQUAL TO THE "T" DIMENSION OF THE BEAM. STANDARD LENGTHS AND AVAILABLE STRENGTH OF CONNECTION ANGLES ARE FOUND IN "A.I.S.C. MANUAL OF STEEL CONSTRUCTION" (13TH EDITION), TABLES 10–1 THRU 10–3.
16. PROVIDE VERTICAL WEB STIFFENERS ON EACH SIDE OF WEB OF BEAM AT ALL POINTS SUBJECTED TO CONCENTRATED LOADS, SUCH AS COLUMN RESTING ON BEAM AND BEAM FRAMING INTO A BEAM. THE STIFFENERS SHALL EXTEND TO FULL DEPTH OF BEAM AND THE BOUNDARY OF FLANGE WITH MINIMUM THICKNESS OF 3/8".
17. NATURAL MILL CAMBER EXISTING IN BEAMS SHALL BE TURNED POSITIVE UPWARD.
18. BURNING OF HOLES IN STRUCTURAL STEEL IS NOT PERMITTED WITHOUT PRIOR APPROVAL OF THE ENGINEER OF RECORD.
19. MAINTAIN WORK IN A SAFE AND STABLE CONDITION DURING ERECTION. PROVIDE TEMPORARY SHORING AND BRACING MEMBERS AS REQUIRED, WITH CONNECTIONS OF SUFFICIENT STRENGTH TO BEAR IMPOSED LOADS. REMOVE TEMPORARY MEMBERS AND CONNECTIONS WHEN PERMANENT MEMBERS ARE IN PLACE AND FINAL CONNECTIONS ARE MADE. PROVIDE TEMPORARY GUY LINES TO ACHIEVE PROPER ALIGNMENT AND STABILITY OF THE STRUCTURE AS ERECTION PROCEEDS.
20. MOMENT CONNECTIONS DESIGNATED BY ► SHALL BE DESIGNED BY THE STEEL SUPPLIER IN ACCORDANCE WITH AISC "MANUAL OF STEEL CONSTRUCTION". THE CONNECTION SHALL BE DESIGNED FOR THE MOMENT CAPACITY OF THE BEAM AND BEAM VERTICAL LOAD CAPACITY.
21. HIGH STRENGTH BOLTED CONNECTIONS AND WELDED CONNECTIONS SHALL BE INSPECTED BY AN INDEPENDENT TESTING LABORATORY. ALL COMPLETE–PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED BY A QUALIFIED INSPECTOR. INSPECTION AND TESTING WILL BE PAID FOR BY THE OWNER.
21. SHOP PRIME ALL STRUCTURAL STEEL (EXCEPT STEEL THAT IS TO RECEIVE FIREPROOFING) WITH STANDARD RED OXIDE PRIMER TO A MINIMUM OF 2 MIL DRY FILM THICKNESS, UNLESS NOTED OTHERWISE. ALL EXTERIOR EXPOSED STRUCTURAL STEEL SHALL BE HOT–DIPPED GALVANIZED. TOUCH–UP ANY DAMAGED GALVANIZING ON EXTERIOR EXPOSED STRUCTURAL STEEL WITH ZINC CHROMATE PAINT CONTAINING A MINIMUM 6% ZINC CHROMATE SOLIDS.

STEEL JOISTS

1. STEEL JOISTS SHALL MEET ALL REQUIREMENTS OF THE S.J.I. AND A.I.S.C. STANDARD SPECIFICATIONS FOR THE TYPE AND SERIES SHOWN.
2. STEEL JOISTS SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH LATEST S.J.I. AND A.I.S.C. SPECIFICATIONS. JOIST DESIGN SHALL BE PERFORMED BY A LICENSED STRUCTURAL ENGINEER.
3. DESIGN OF STEEL JOISTS AND THEIR CONNECTIONS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE THE JOISTS ARE TO BE ERECTED INDICATING THE MANUFACTURER'S NAME, JOIST LAYOUT, AND DETAILS. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE STEEL JOISTS AND THEIR CONNECTIONS.
4. STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS FOR A NET UPLIFT (DUE TO WIND LOADING) OF 15 PSF. DIAGONAL BRIDGING OR BRACING TO LATERALLY BRACE THE BOTTOM CHORD SHALL BE PROVIDED AS REQUIRED.
5. STEEL JOISTS DESIGNATED "SPECIAL" (SPECIAL, NON–STANDARD) SHALL BE DESIGNED BY THE MANUFACTURER FOR THE LOADS INDICATED ON THE DRAWINGS. DESIGN SHALL CONFORM TO AISC AND SJI STANDARD SPECIFICATIONS AND SHALL BE PERFORMED BY A REGISTERED PROFESSIONAL STRUCTURAL ENGINEER.
6. BRIDGING SHALL BE FURNISHED AND INSTALLED AS REQUIRED BY THE A.I.S.C. AND S.J.I. STANDARD SPECIFICATIONS AND/OR AS INDICATED ON PLANS. BRIDGE JOIST IMMEDIATELY AFTER ERECTION AND BEFORE CONSTRUCTION LOADS ARE APPLIED.
7. THE ENDS OF BRIDGING LINES TERMINATING AT MASONRY WALLS SHALL BE ANCHORED BY STRAP ANCHORS ATTACHED TO THE WALL UNLESS OTHERWISE SHOWN OR NOTED.
8. FURNISH AND INSTALL BOTTOM AND TOP CHORD LATERAL BRACING AS REQUIRED FOR STRENGTH AND STABILITY OF JOISTS AND JOIST GIRDERS.
9. ENDS OF STEEL JOISTS SHALL BE ANCHORED TO THE STEEL SUPPORTS BY WELDING.

METAL ROOF DECK

1. 1 1/2 INCH ROOF DECK SHALL BE 20 GAUGE GAUGE GALVANIZED (G90) WITH A FABRICATED DEPTH OF 1 1/2 INCHES AND A VALLEY SPACING OF 6 INCHES. DECKING SHALL BE FACTORY GALVANIZED. DECKING SHALL CONFORM TO ASTM A653–94 STRUCTURAL QUALITY GRADE 33 OR HIGHER. DECKING SHALL BE FASTENED TO STEEL SUPPORTING MEMBERS AT 12 INCHES ON CENTER, MAXIMUM SPACING, WITH NO. 12 SELF–TAPPING METAL SCREW. SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED WITH NO. 10 METAL SCREWS, AT 12 INCHES ON CENTER, MAXIMUM SPACING. SHEETS SHALL BE CONTINUOUS FOR AT LEAST THREE SPANS WHERE POSSIBLE.
2. COMPLY WITH THE PROVISIONS OF THE LATEST EDITIONS OF THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS, EXCEPT AS OTHERWISE SHOWN OR SPECIFIED.
   A. A.I.S.I. "SPECIFICATIONS FOR THE DESIGN OF COLD–FORMED STEEL STRUCTURAL MEMBERS".
   B. AWS D1.3 "STRUCTURAL WELDING CODE".
   C. SDI "STEEL DECK DESIGN MANUAL".
3. ACCESSORIES SHALL BE STANDARD WITH THE MANUFACTURER AND SHALL BE FURNISHED AS NECESSARY TO COMPLETE THE ROOF DECK INSTALLATION.
4. ATTENTION IS CALLED TO THE FACT THAT THE METAL ROOF DECK IS DESIGNED FOR DIAPHRAGM ACTION. THEREFORE, ADDED CARE MUST BE TAKEN TO ASSURE PROPER INSTALLATION PROCEDURES ARE FOLLOWED.
5. A TESTING COMPANY SHALL BE RETAINED TO ENSURE THAT THE DECK IS FASTENED PROPERLY PRIOR TO PLACEMENT OF COVER MATERIALS. WRITTEN APPROVAL OF DECK INSTALLATION MUST BE SUBMITTED TO THE ARCHITECT/ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

CONTRACTOR RESPONSIBILITIES

1. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
2. COORDINATE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.
3. VERIFY THE DIMENSIONS, ELEVATIONS AND SITE CONDITIONS BEFORE STARTING WORK. ANY DISCREPANCY BETWEEN SUCH DETAILS AND DIMENSIONS AS MAY OCCUR SHALL BE REPORTED TO THE ARCHITECT/ENGINEER FOR CLARIFICATION BEFORE PROCEEDING WITH THE WORK.
4. NOTIFY, IN WRITING, THE STRUCTURAL ENGINEER OF CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN IN THE STRUCTURAL DOCUMENTS.
5. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
6. CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC.
7. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA SAFETY REGULATIONS.
8. SUBMITTALS

   FURNISH ONE HARD COPY & ONE ELECTRONIC COPY OF ALL SHOP DRAWINGS AND SUBMITTALS. THE HARD COPY WILL BE RETAINED BY THE ENGINEER. THE ELECTRONIC COPY WILL BE RETURNED WITH COMMENTS.

   REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.

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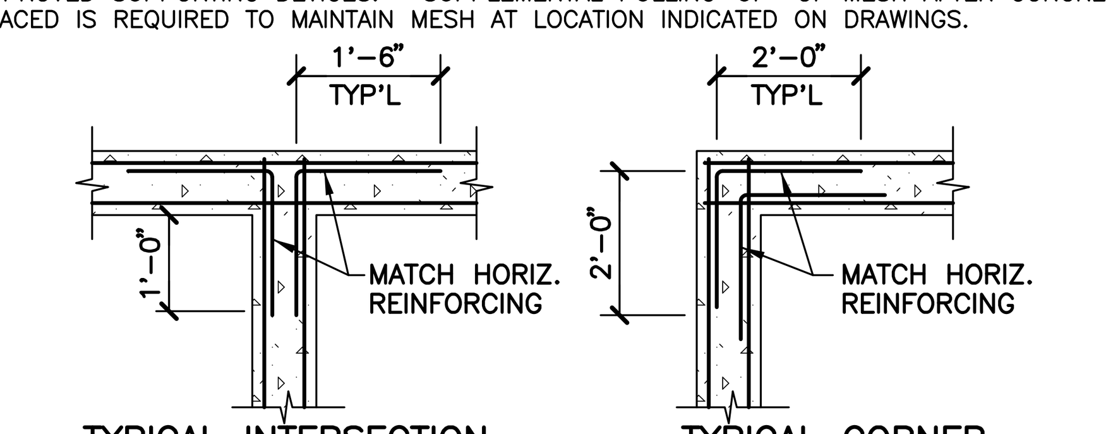
LOUISVILLE, KENTUCKY 40222

MISCELLANEOUS

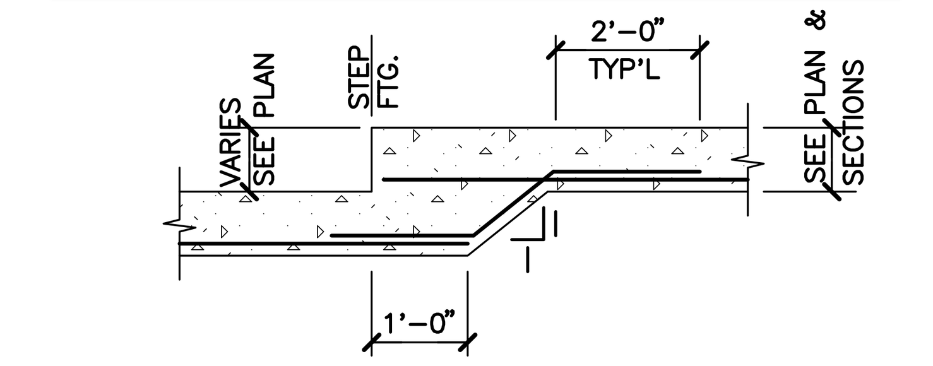
1. SHRINKAGE–RESISTING GROUT FOR USE BENEATH COLUMN BASEPLATES AND BEAM BEARINGS SHALL BE PRE–MIXED, FACTORY PACKAGED, NON–STAINING, NON–METALLIC, NON–GASING MORTAR GROUTING COMPOUND, COMPLYING WITH THE REQUIREMENTS OF A.S.T.M. C1107. GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 6,000 PSI.
2. MATERIAL FOR USE AS VAPOR BARRIER BENEATH CONCRETE SLABS ON GRADE SHALL BE 15 MIL POLYETHYLENE SHEETS, COMPLYING WITH ASTM D–2103. SHEETS SHALL BE LAPPED A MINIMUM OF 6" AT ALL EDGES. SPECIAL CARE SHALL BE TAKEN TO PREVENT PUNCTURING SHEETS PRIOR TO PLACEMENT OF SLABS.
3. NO CHANGE IN SIZE OF STRUCTURAL ELEMENTS OR MODIFICATION THEREOF SHALL BE MADE, NOR ARE ANY OPENINGS OR SLEEVES THROUGH ANY STRUCTURAL ELEMENTS PERMITTED, UNLESS DETAILED ON THE DRAWINGS.
4. CONSULT ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION, SIZES AND EXTENT OF CHASES, INSERTS, RECESSES, REGELETS, FINISHES, DEPRESSIONS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.
5. ALL WELDED WIRE FABRIC IN SLABS ON GRADE SHALL BE SUPPORTED BY CHAIRS, BOLSTERS, OR OTHER APPROVED SUPPORTING DEVICES. "SUPPLEMENTAL PULLING–UP" OF MESH AFTER CONCRETE HAS BEEN PLACED IS REQUIRED TO MAINTAIN MESH AT LOCATION INDICATED ON DRAWINGS.



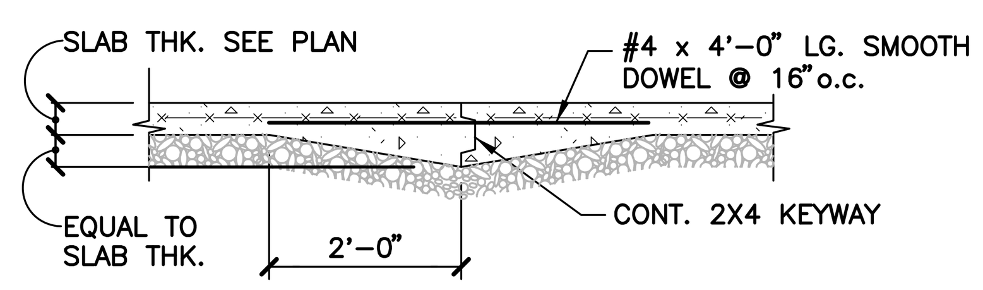
TYPICAL INTERSECTION WALL/BOND BEAM/FOOTING

TYPICAL CORNER WALL/BOND BEAM/FOOTING

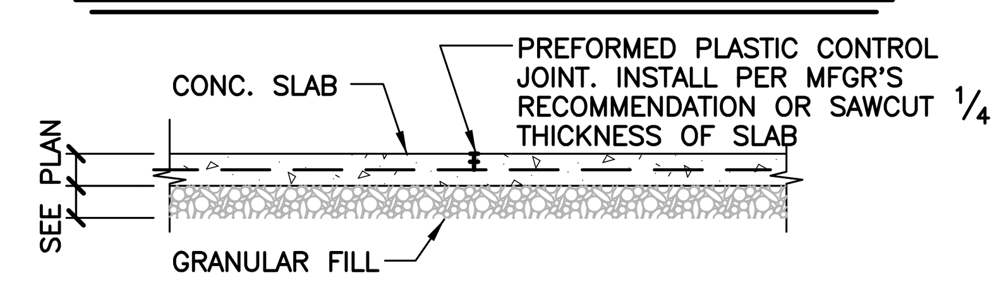
TYPICAL CORNER BAR DETAILS



TYPICAL STEP FTG. DETAIL



TYPICAL SLAB ON GRADE CONSTRUCTION JOINT DETAIL



TYPICAL SLAB ON GRADE CONTROL JOINT DETAIL

CFDS003B

REINFORCEMENT LAP SPLICE LENGTH
(GRADE 60 REINFORCEMENT)

THE FOLLOWING LAP SPLICE SHALL BE PROVIDED FOR REINFORCEMENT; UNLESS NOTED OTHERWISE

	4000 PSI CONCRETE		MASONRY
	TOP BARS	OTHERS	
#3	24"	19"	18"
#4	32"	25"	24"
#5	40"	31"	30"
#6	48"	37"	36"
#7	70"	54"	42"
#8	80"	62"	48"
#9	91"	70"	55"
#10	102"	79"	––
#11	113"	87"	––

(TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 IN. OF CONCRETE CAST BELOW THE BARS)

CFED007Q

NOTE: ALL WELDED WIRE FABRIC LAP SPLICES SHALL BE ONE SPACE PLUS 2" (i.e. WWF 6x6 = 8" SPLICE)

CONCRETE PROTECTION FOR REINFORCEMENT IN CAST–IN–PLACE CONCRETE

THE FOLLOWING CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT; UNLESS NOTED OTHERWISE

COVER, IN.

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3

CONCRETE EXPOSED TO EARTH OR WEATHER
- #6 THROUGH #18 BARS 2
- #5 BAR, W31 OR D31 WIRE, AND SMALLER 1 1/2

CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
- SLABS, WALLS, JOIST:
  - #14 AND #18 BARS 1 1/2
  - #11 BAR AND SMALLER 3/4
- BEAMS AND COLUMNS:
  - PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS 1 1/2

CFED007A

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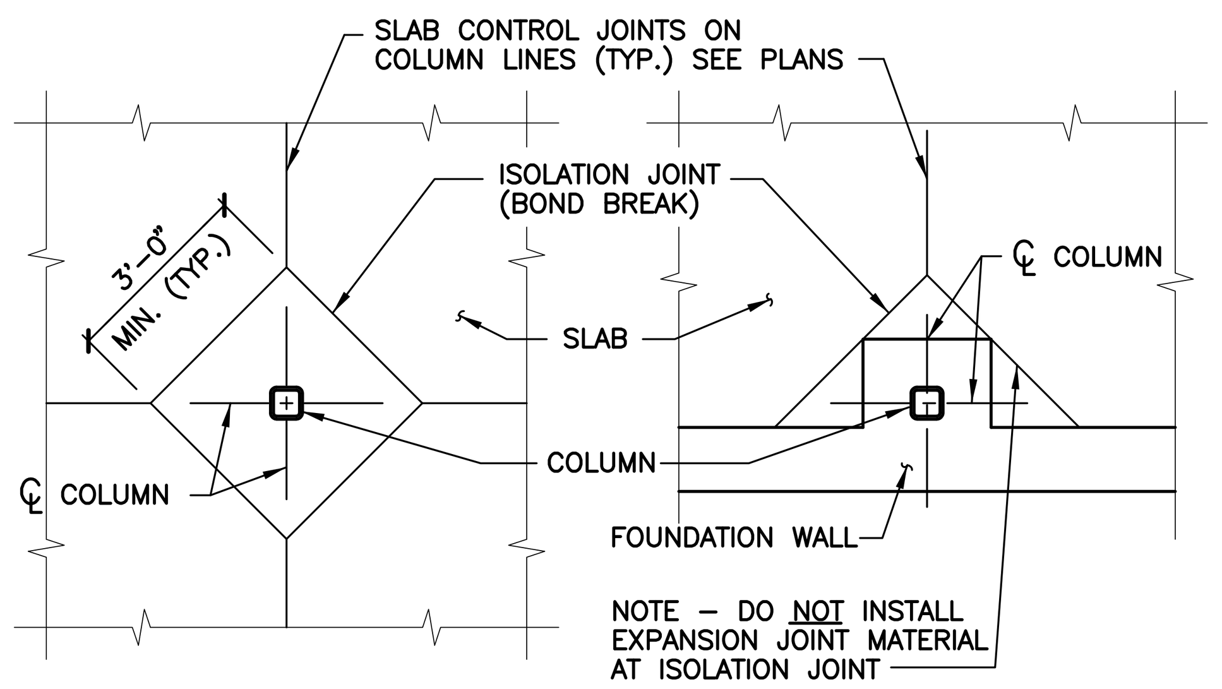
Town of Clarksville

New Facility

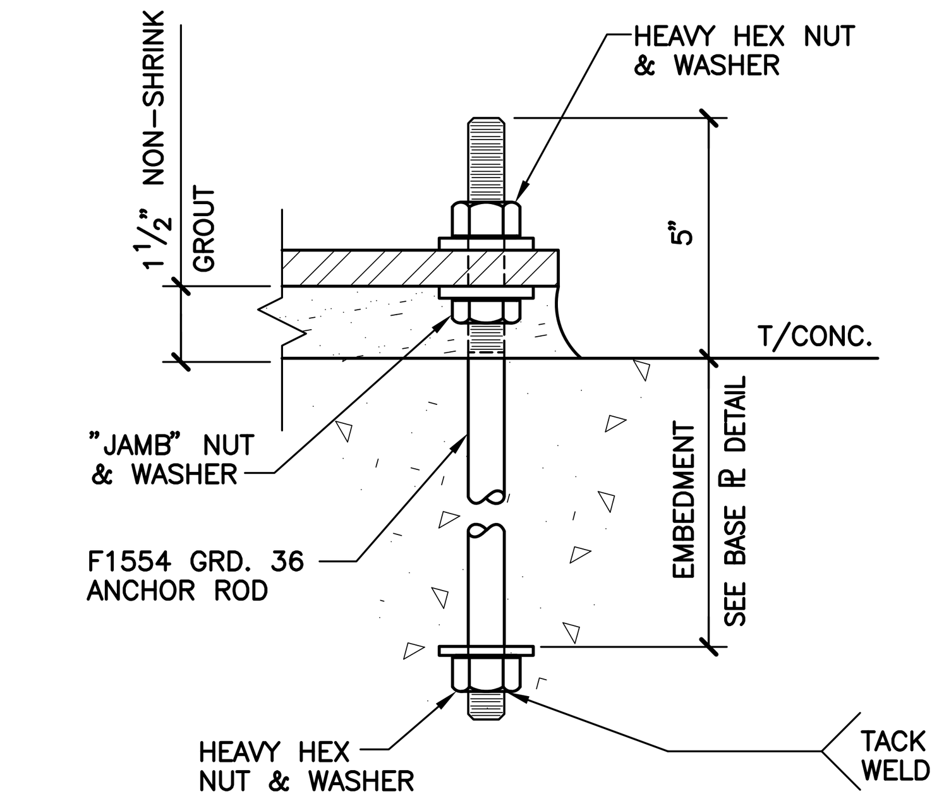
Sheet
S-001



TYPICAL ISOLATION JOINT IN SLAB ON GRADE AT COLUMN

CFDD018N

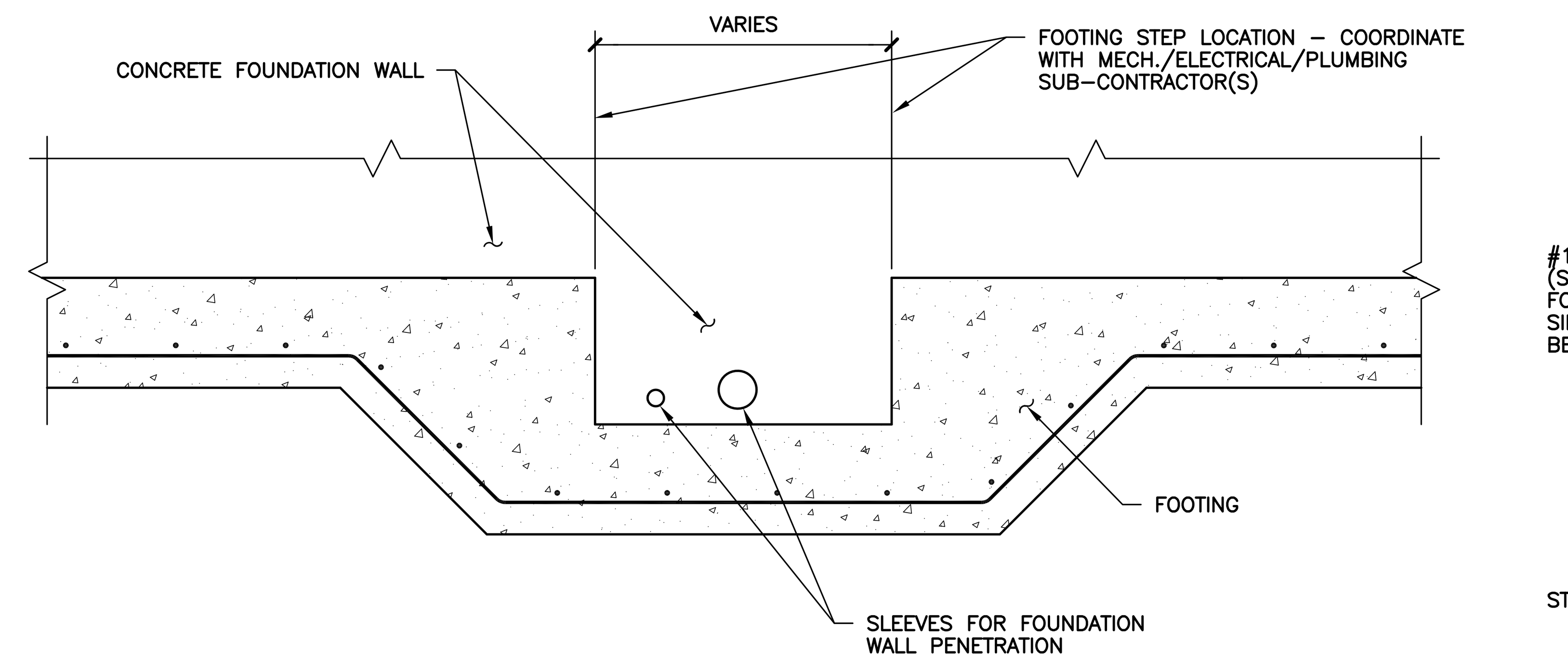
NO SCALE



TYPICAL ANCHOR ROD DETAIL

SCALE: 3" = 1'-0

SBHD012C



TYPICAL STEP FOOTING DETAIL @ M E P PENETRATION

CFES1094B

1. GENERAL CONTRACTOR SHALL COORDINATE ALL INTERIOR ISOLATED & STRIP FOOTING STEPS WITH M E P LAYOUT AND SUB-CONTRACTOR(S).

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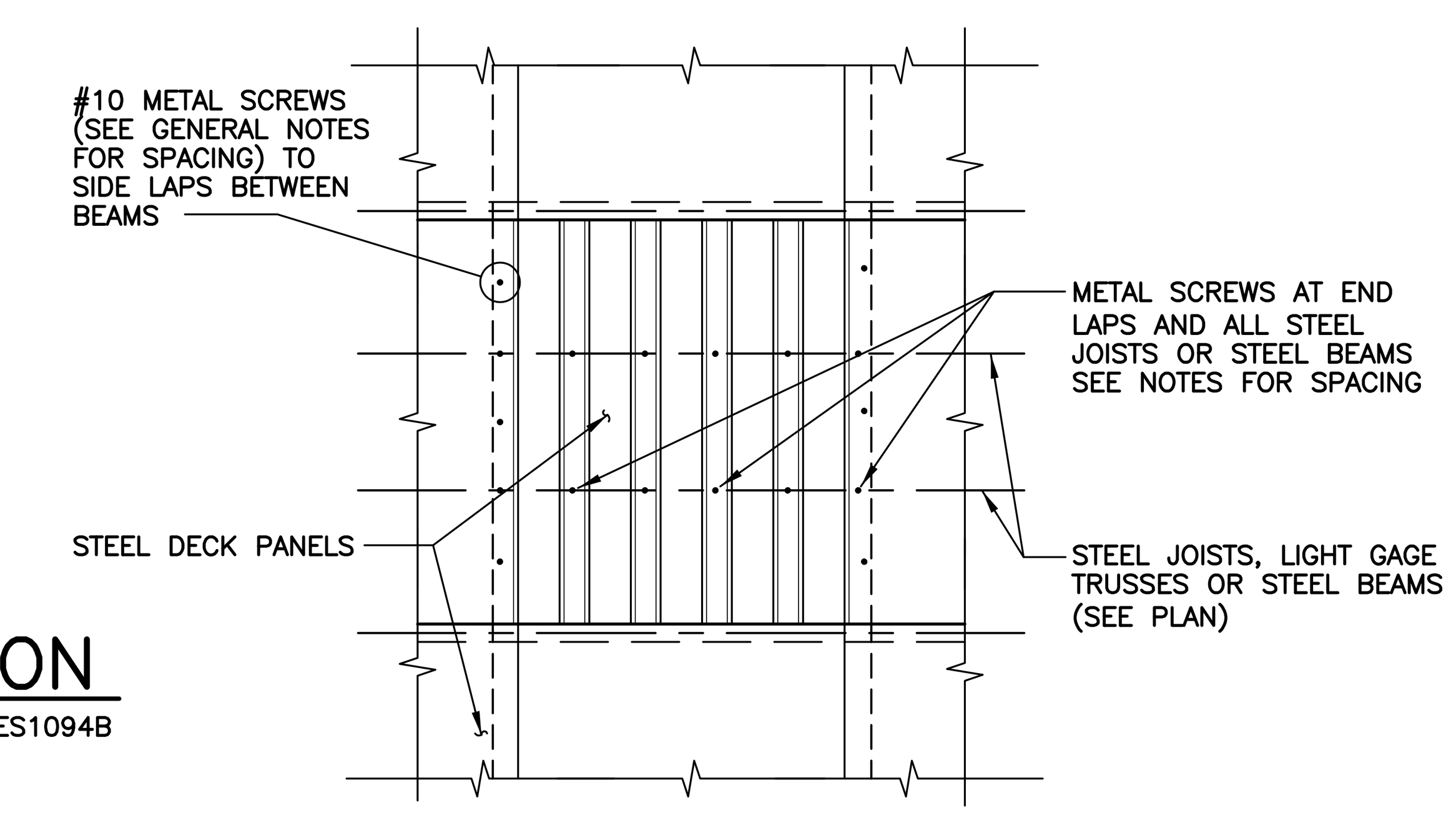
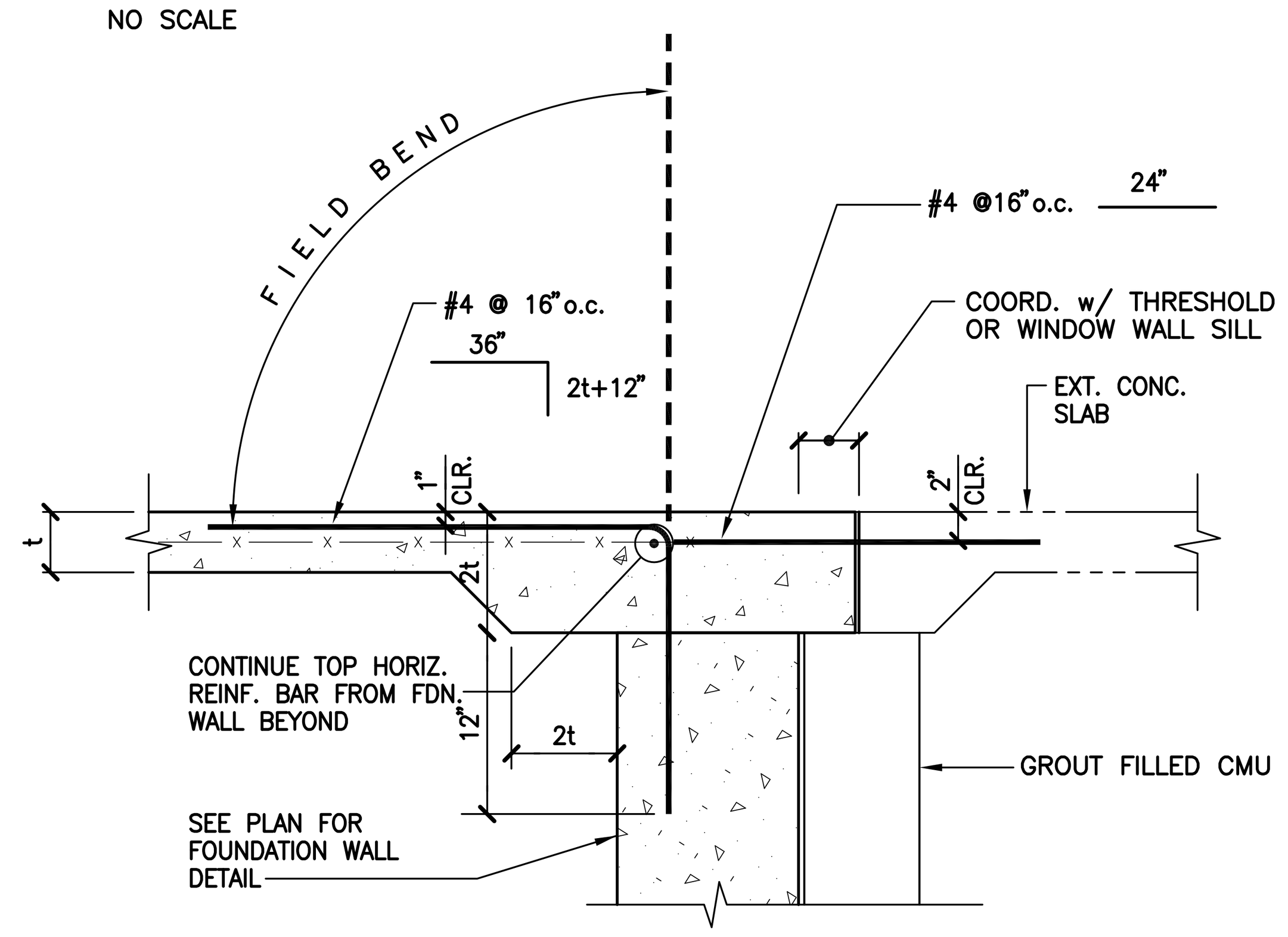


TYPICAL FASTENING PATTERN FOR STEEL DECK PANELS

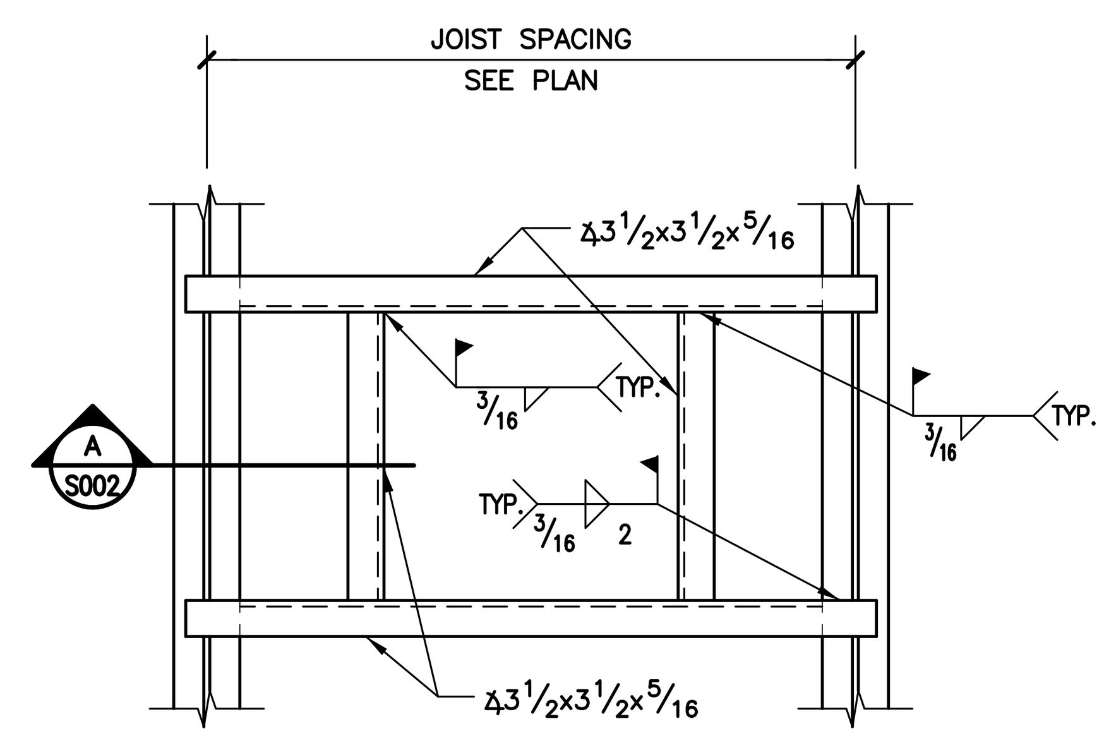
SRED016B



TYPICAL THICKENED SLAB @ EXT. DOORWAY

SCALE: 1" = 1'-0"

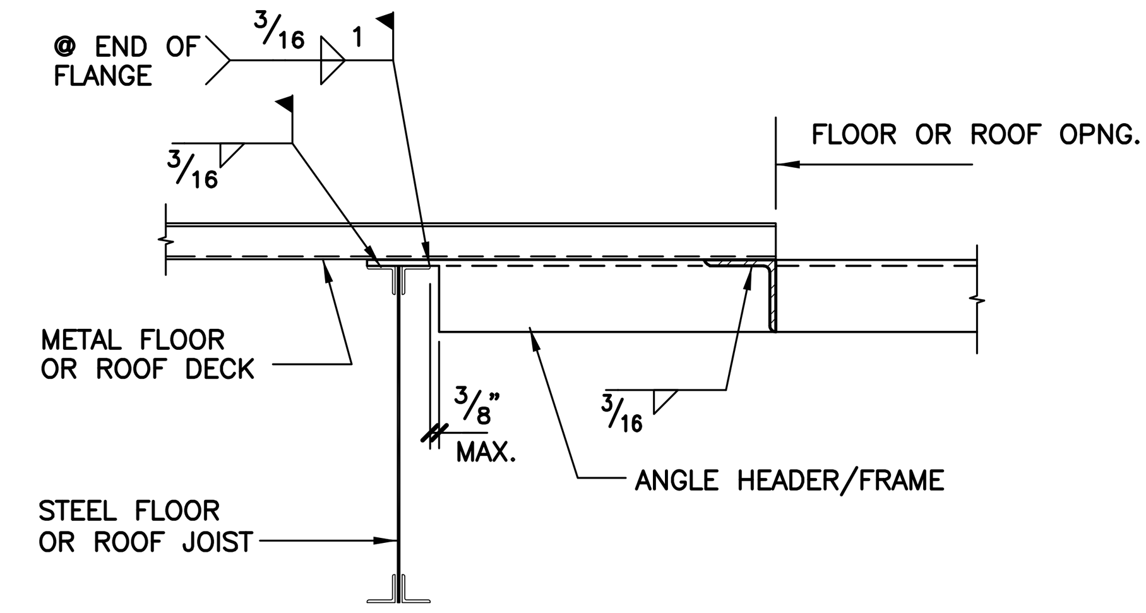
CFFS024R



COORDINATE SIZE AND LOCATION WITH ARCH'L & MECH'L DRAWINGS

TYPICAL FLOOR OR ROOF OPENING FRAME

SRED003L



SECTION

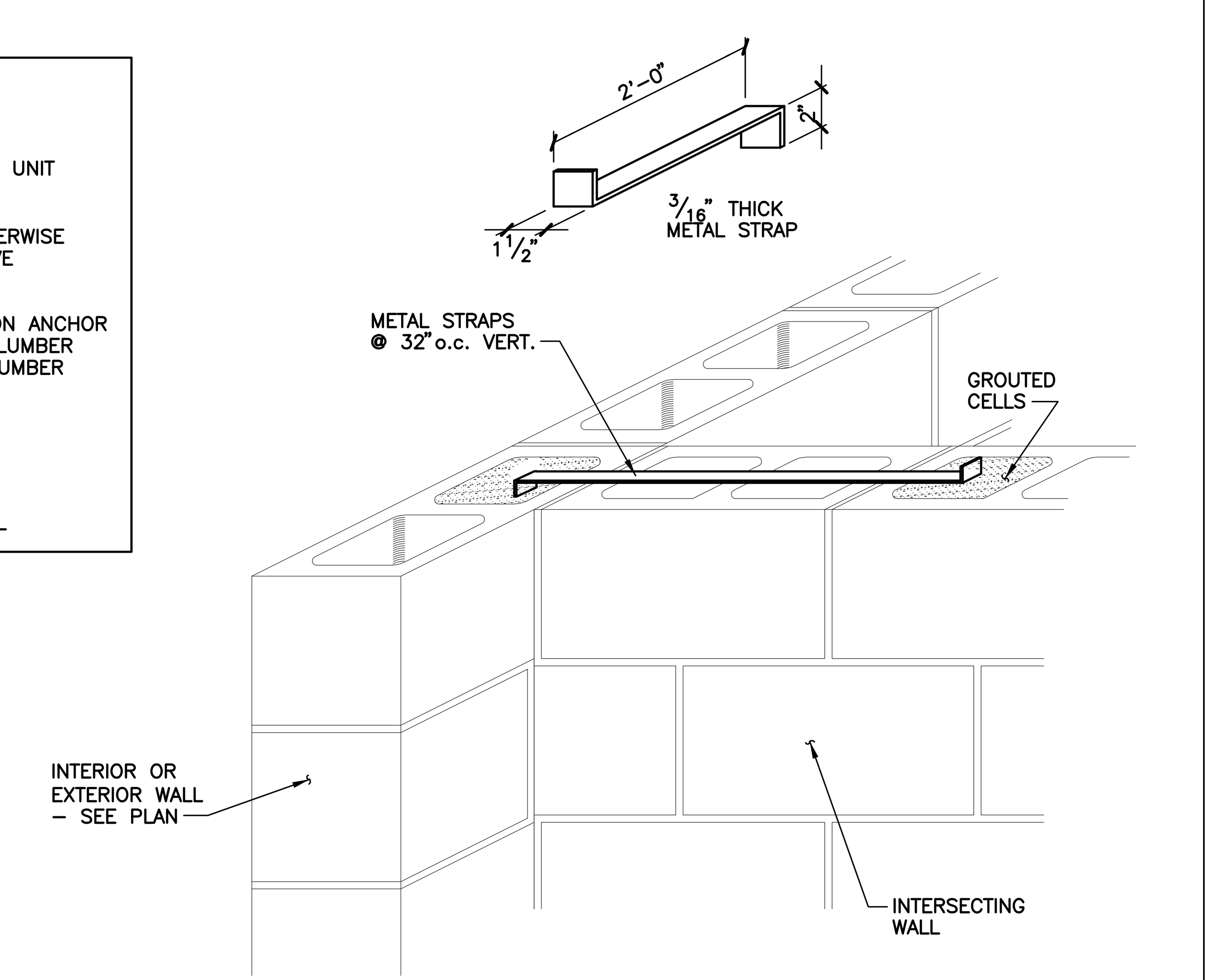
SCALE: 1 1/2" = 1'-0

SRGS059C

A S002

LEGEND & ABBREVIATIONS

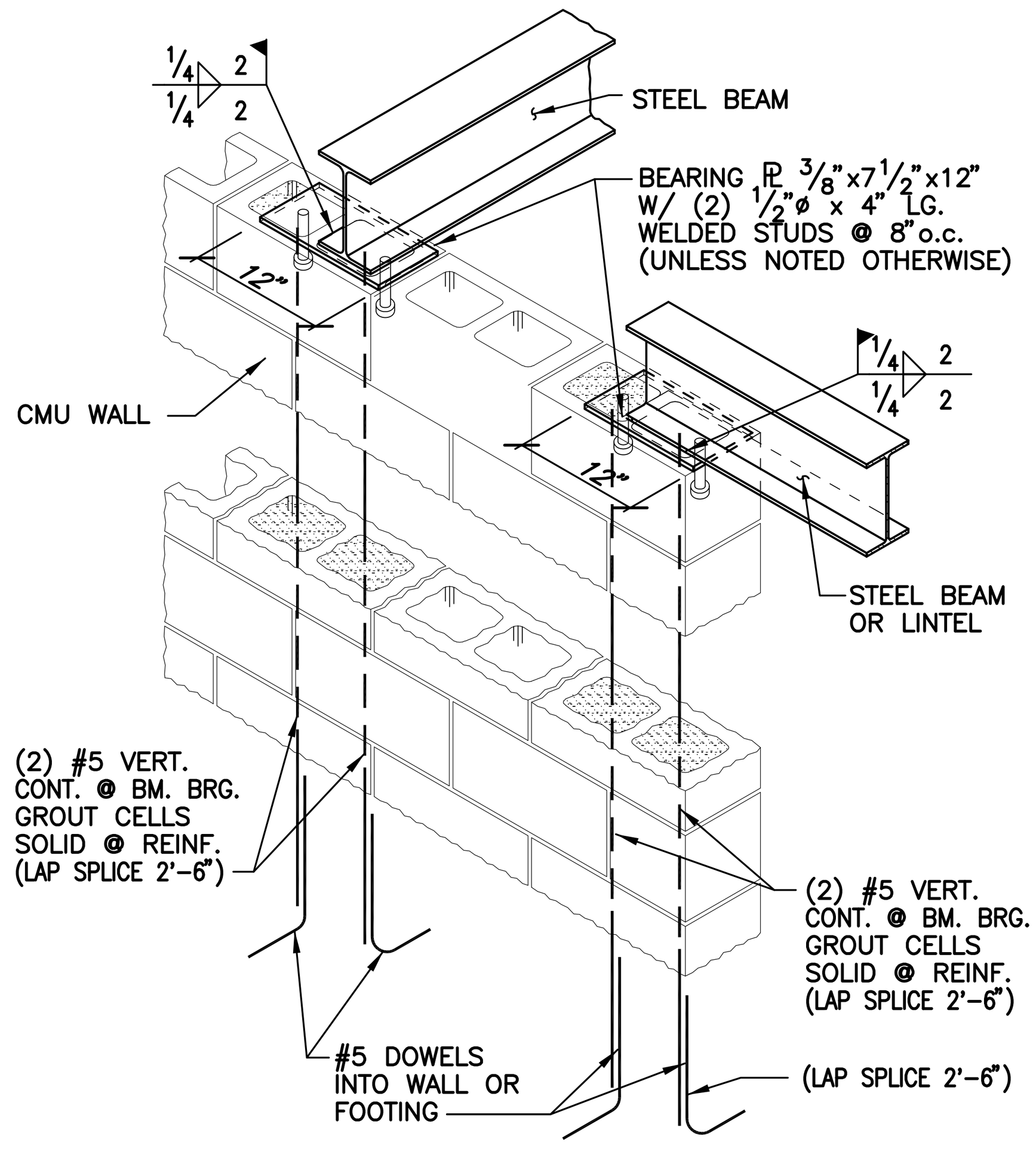
TYP =	TYPICAL
N.S. =	NEAR SIDE
F.S. =	FAR SIDE
O.C. =	ON CENTER
CMU =	CONCRETE MASONRY UNIT
T/ =	TOP OF
CLR =	CLEAR
U.N.O.=	UNLESS NOTED OTHERWISE
T&G =	TONGUE AND GROOVE
P.T. =	PRESSURE TREATED
KB3 =	HILTI KWIK BOLT 3
CEA =	CONCRETE EXPANSION ANCHOR
LVL =	LAMINATED VENEER LUMBER
PSL =	PARALLEL STRAND LUMBER
GT =	GIRDER TRUSS
ALT =	ALTERNATE
EA =	EACH
W/ =	WITH
CLR COV =	CLEAR COVER
HT =	HEADER TRUSS
CFS =	COLD FORMED STEEL



METAL STRAP DETAIL @ INTERSECTING CMU WALLS

NOT TO SCALE

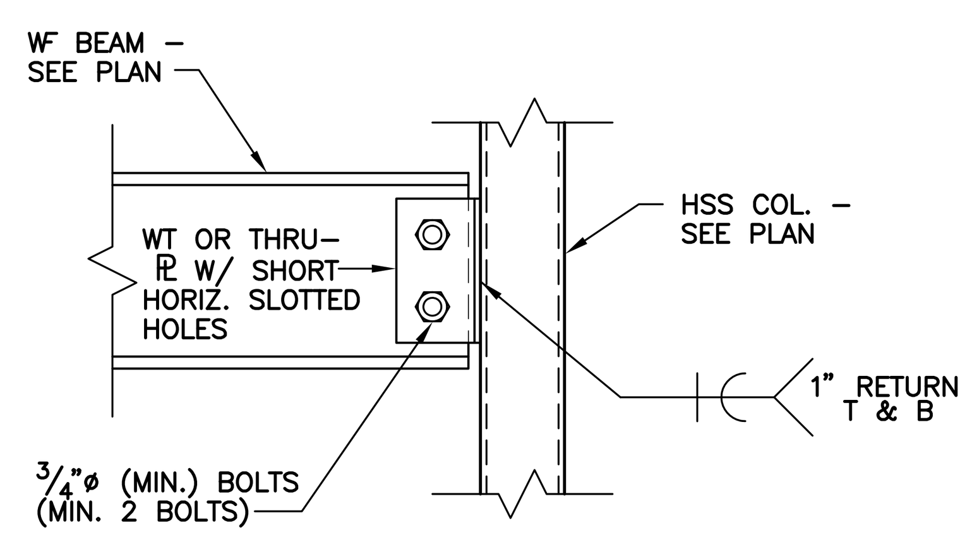
MOED005E



TYPICAL DETAIL @ BEAM BRG.

SCALE: 3/4" = 1'-0 (UNLESS OTHERWISE NOTED)

MOED002R

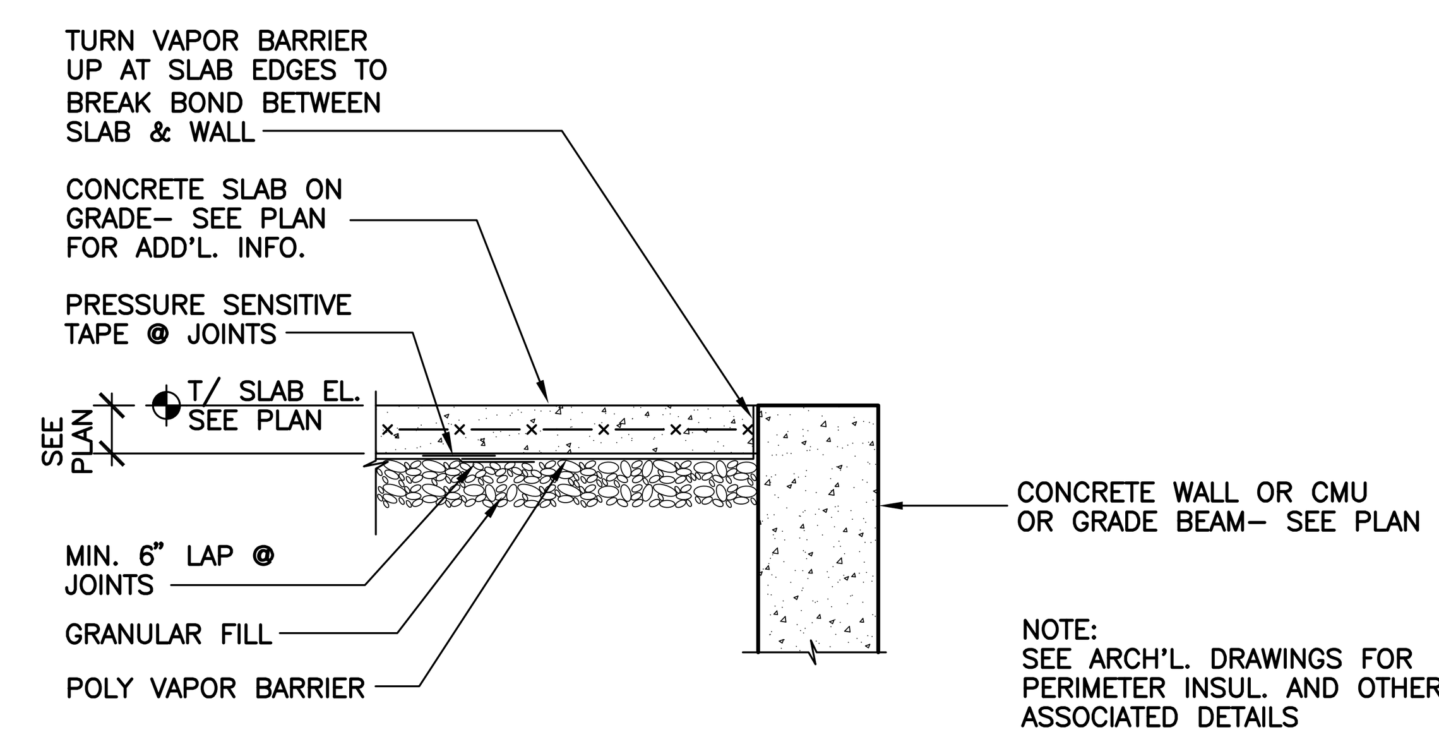


CONNECTIONS TO BE DESIGNED BY STRUCTURAL STEEL SUPPLIER IN ACCORDANCE WITH GENERAL NOTES

TYP. BEAM TO HSS COL. CONN.

SCALE: 1 1/2" = 1'-0

SOGS043A

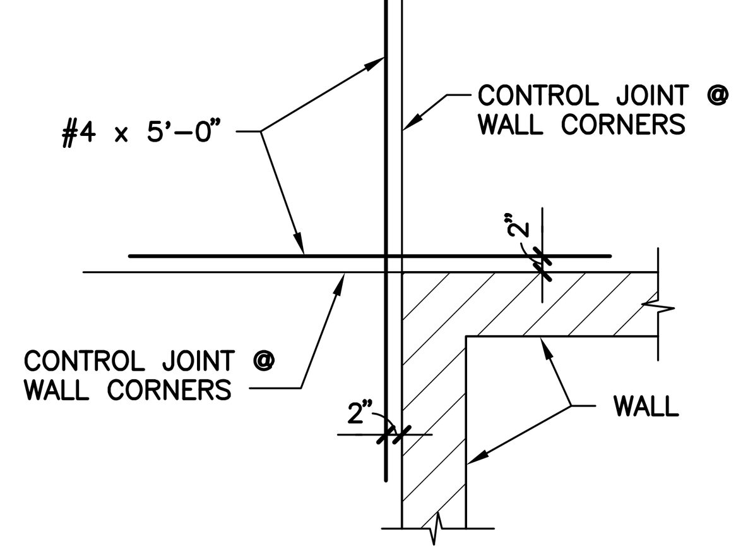


NOTE:
SEE ARCH'L. DRAWINGS FOR PERIMETER INSUL. AND OTHER ASSOCIATED DETAILS

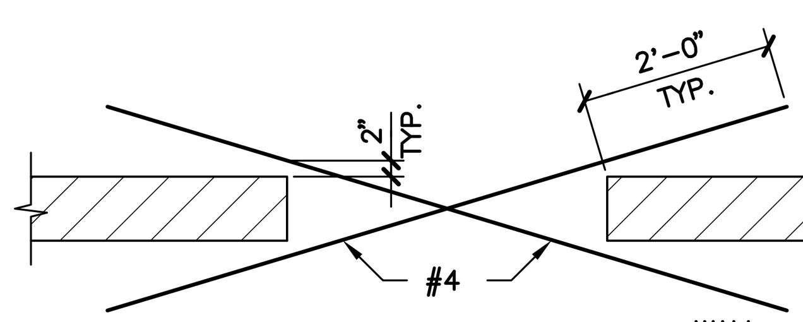
TYP. WALL/SLAB EDGE

SCALE: 3/4" = 1'-0

CFES2262H



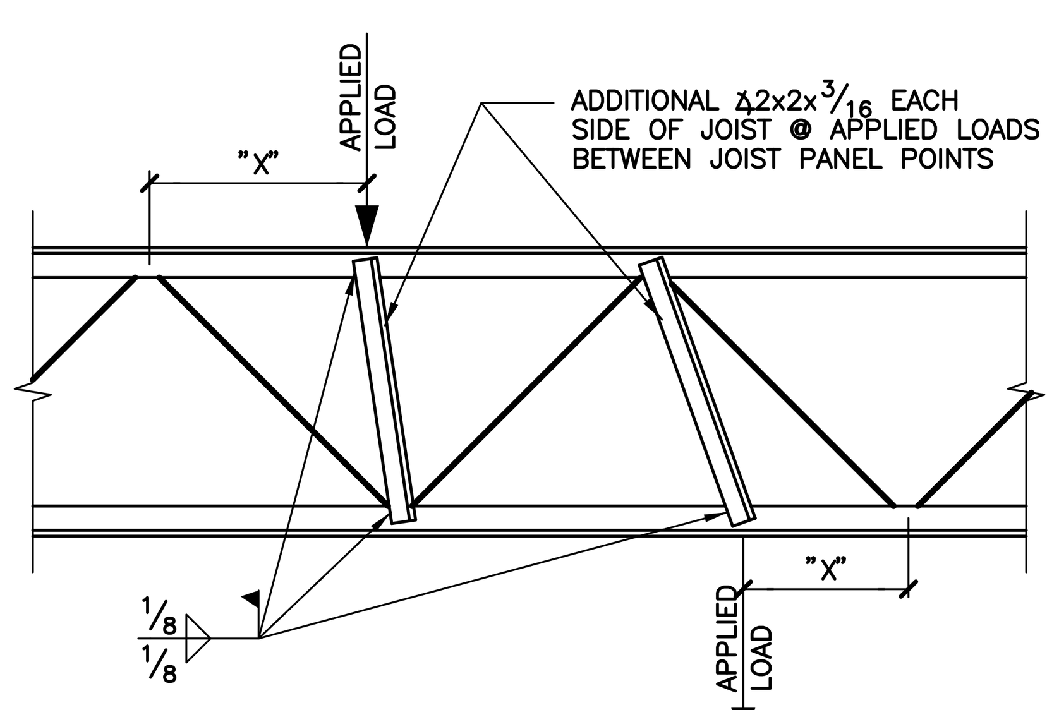
TYPICAL WALL CORNER



TYPICAL INTERIOR DOOR OPENING

TYPICAL SLAB REINFORCING DETAILS

CFDP067

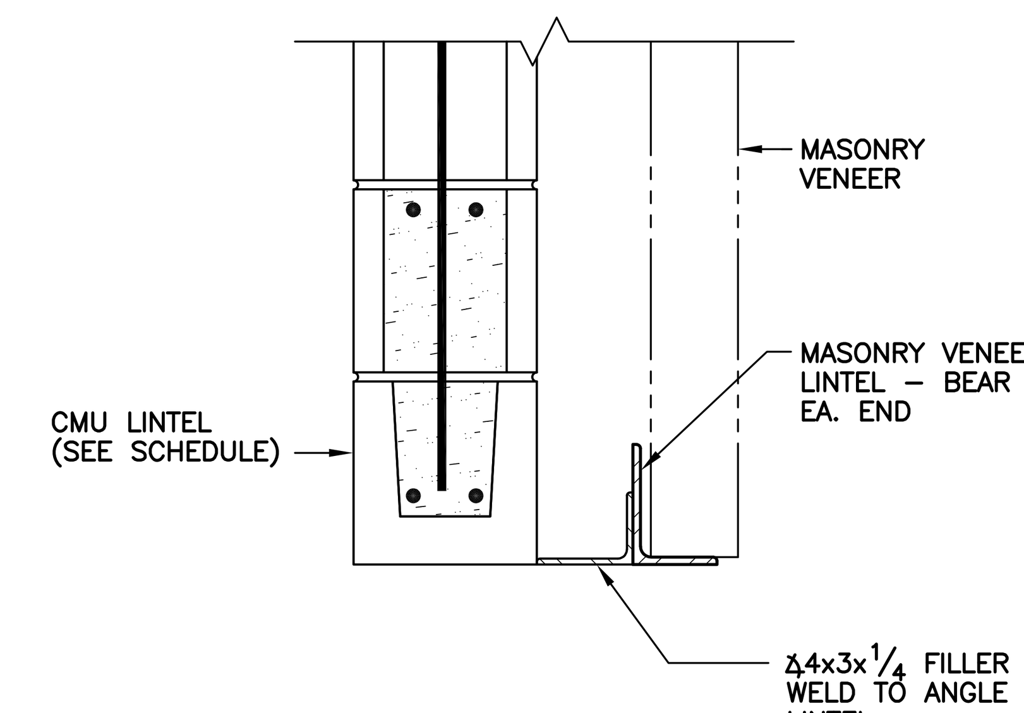


NOTE: ∠2x2 REQUIRED WHERE "X" IS GREATER THAN 6"

TYPICAL STEEL JOIST CHORD STIFFENER @ APPLIED LOADS

SCALE: NONE

SRES008F



TYPICAL LINTEL DETAIL

SCALE: 1 1/2" = 1'-0

MNES016B

LINTEL SCHEDULE

MARK NO.	CMU	MASONRY VENEER
L1	8" DEEP BOND BEAM w/ (2)#5 BOT.	∠3 1/2x3 1/2x5/16
L2	24" DEEP BOND BEAM w/ (2)#5 TOP & BOT.	∠5x3 1/2x5/16
L3	16" DEEP BOND BEAM w/ (2)#5 TOP & BOT.	∠5x3 1/2x5/16
L4	8" DEEP (12" CMU) BOND BEAM w/ (2)#5 BOT.	---
L5	16" DEEP (12" CMU) BOND BEAM w/ (2)#5 TOP & BOT.	---
L6	(2)∠4x3 1/2x5/16 WELD TO COLUMN @ ONE END	∠4x3 1/2x5/16

ML – MISCELLANEOUS LINTEL SCHEDULE

(USE THIS SCHEDULE FOR LINTELS NOT OTHERWISE SHOWN OR NOTED AND THOSE DESIGNATED ML)

BLOCK LINTELS– FOR CMU WALL
3'-0" WIDE AND LESS – USE 8 INCH DEEP BOND BEAM WITH (2) #5 AT BOTTOM
3'-1" WIDE TO 7'-0" WIDE – USE 16 INCH DEEP BOND BEAM WITH (2) #5 TOP & BOTTOM
7'-1' WIDE TO 11'-0" WIDE – USE 16 INCH DEEP BOND BEAM WITH (2) #6 TOP & BOTTOM

STEEL LINTELS– FOR MASONRY VENEER
4'-0" WIDE OR LESS – USE ANGLE 4X3 1/2X5/16 WITH 8" BEARING EACH END
4'-1" WIDE TO 6'-0" WIDE – USE ANGLE 5X3 1/2 X5/16 WITH 8" BEARING EACH END
6'-1" WIDE TO 9'-0" WIDE – USE ANGLE 6X3 1/2 X 5/16 WITH 12" BEARING EACH END

PROVIDE HOT DIPPED GALVANIZED STEEL WHERE LINTELS ARE EXPOSED TO WEATHER. COORDINATE WITH ARCHITECTURAL DRAWINGS AND DETAILS.

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Date	11/28/17

Revisions

Certified By [illegible] 11/28/17

CARL J. SLESSER
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PROFESSIONAL ENGINEER

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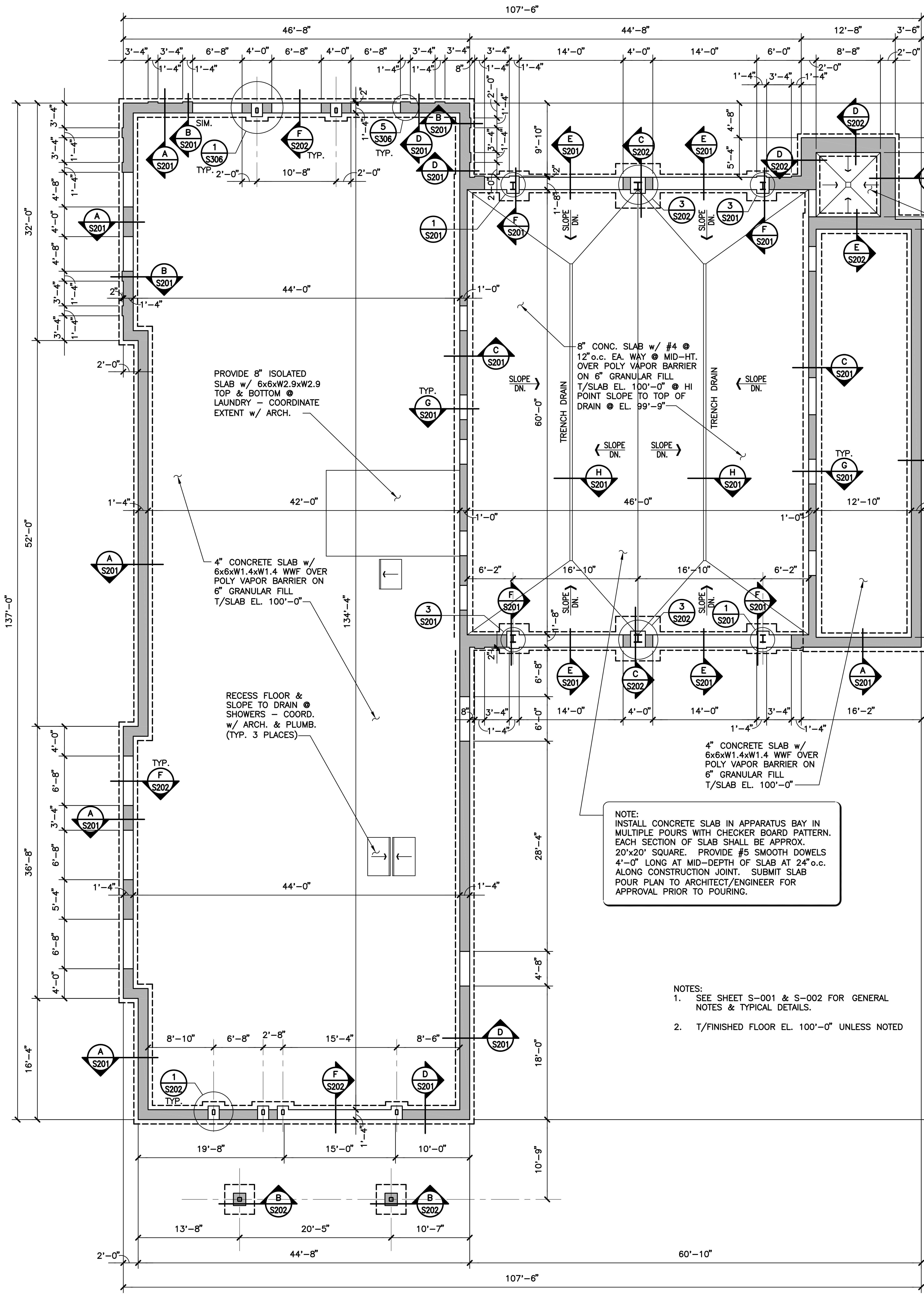
Town of Clarksville

New Facility

Sheet
S-002


PROVIDE 8" ISOLATED SLAB w/ 6x6xW2.9xW2.9 TOP & BOTTOM @ LAUNDRY – COORDINATE EXTENT w/ ARCH.
4" CONCRETE SLAB w/ 6x6xW1.4xW1.4 WWF OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL T/SLAB EL. 100'–0"
RECESS FLOOR & SLOPE TO DRAIN @ SHOWERS – COORD. w/ ARCH. & PLUMB. (TYP. 3 PLACES)
8" CONC. SLAB w/ #4 @ 12"o.c. EA. WAY @ MID–HT. OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL T/SLAB EL. 100'–0" @ HI POINT SLOPE TO TOP OF DRAIN @ EL. 99'–9"
8" CONCRETE SLAB w/ 6x6xW2.9xW2.9 WWF OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL – T/SLAB EL. 100'–0" @ PERIMETER SLOPE TO TOP OF DRAIN @ EL. 99'–9"
4" CONCRETE SLAB w/ 6x6xW1.4xW1.4 WWF OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL T/SLAB EL. 100'–0"
TRENCH DRAIN
SLOPE DN.
NOTE:
INSTALL CONCRETE SLAB IN APPARATUS BAY IN MULTIPLE POURS WITH CHECKER BOARD PATTERN. EACH SECTION OF SLAB SHALL BE APPROX. 20'x20' SQUARE. PROVIDE #5 SMOOTH DOWELS 4'–0" LONG AT MID–DEPTH OF SLAB AT 24"o.c. ALONG CONSTRUCTION JOINT. SUBMIT SLAB POUR PLAN TO ARCHITECT/ENGINEER FOR APPROVAL PRIOR TO POURING.
107'–6"
46'–8"
44'–8"
12'–8"
3'–6"
137'–0"
134'–4"
60'–10"
NORTH

NOTES:
1. SEE SHEET S–001 & S–002 FOR GENERAL NOTES & TYPICAL DETAILS.
2. T/FINISHED FLOOR EL. 100'–0" UNLESS NOTED

FOUNDATION PLAN
SCALE: 1/8" = 1'–0"

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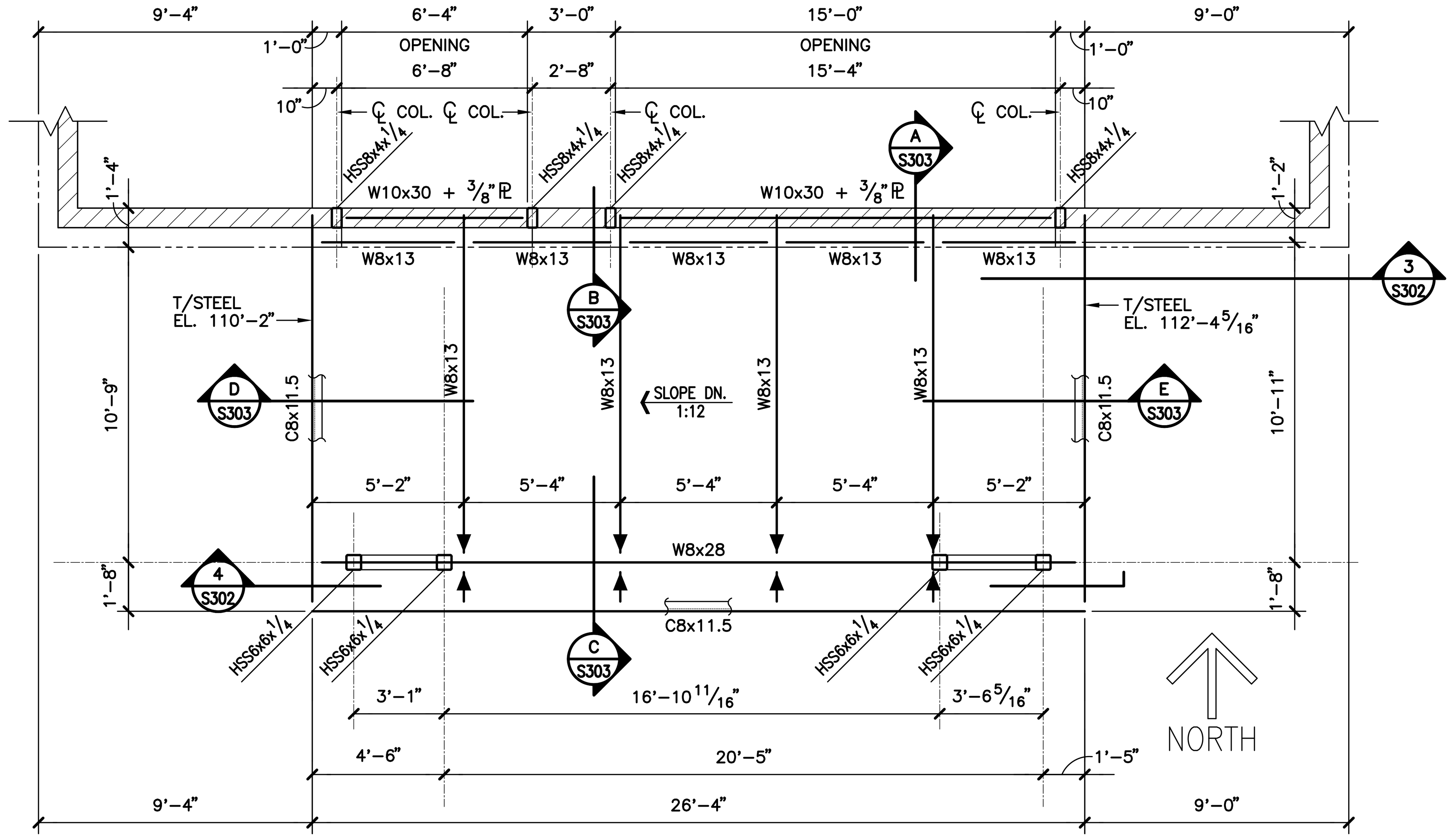
New Facility

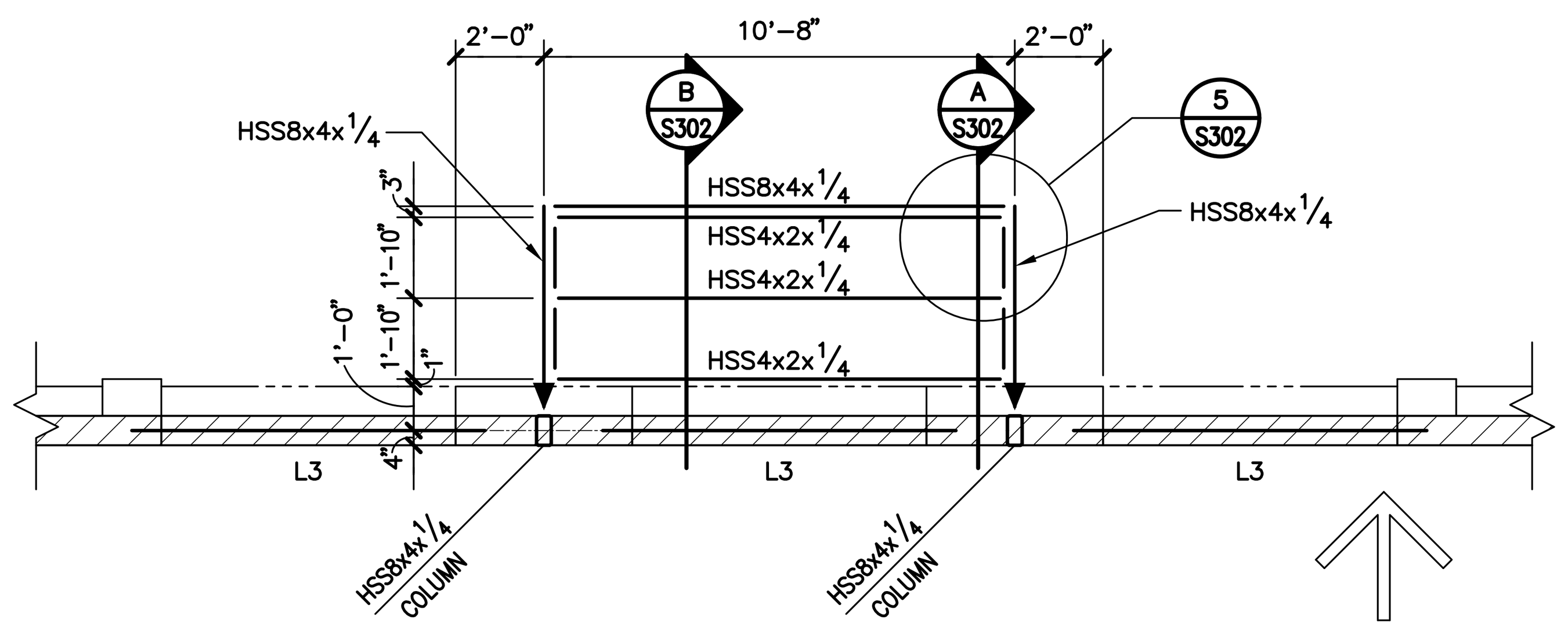
Sheet
S-101



CANOPY ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0

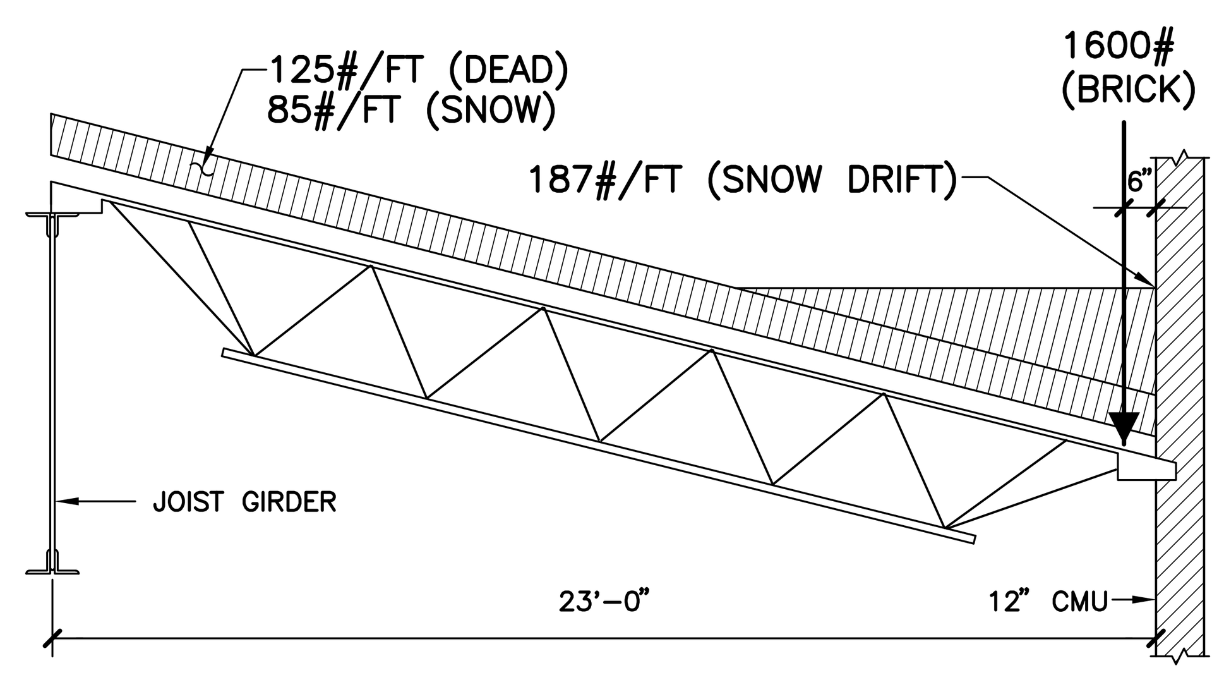
SRBP094A 1 S102



CANOPY ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0

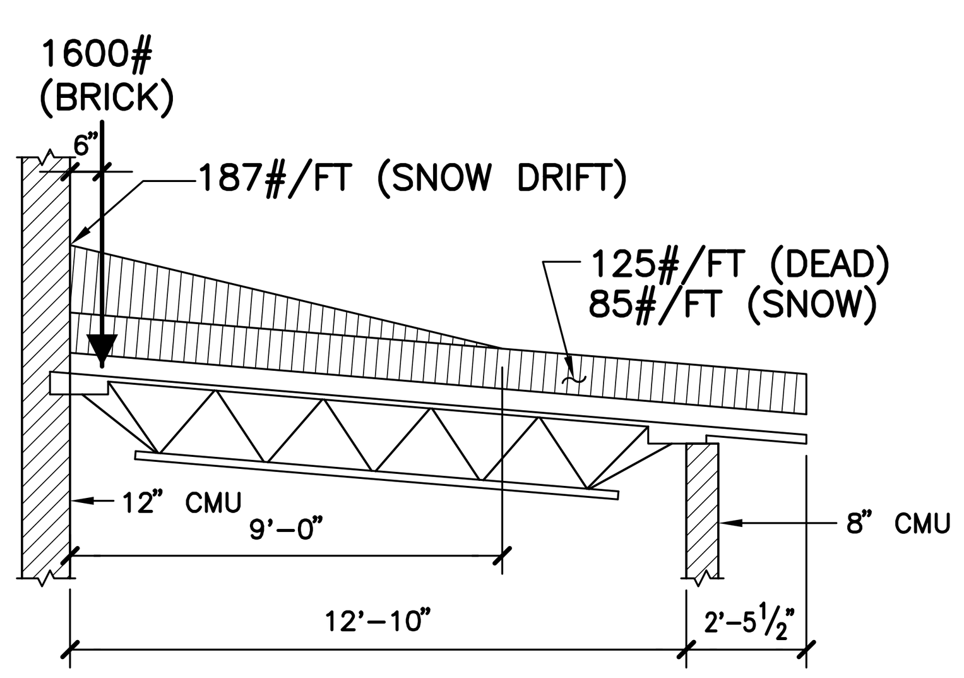
SRBP094B 2 S102



16KSP1 SPECIAL JOIST DIAGRAM

SCALE: 1/4" = 1'-0

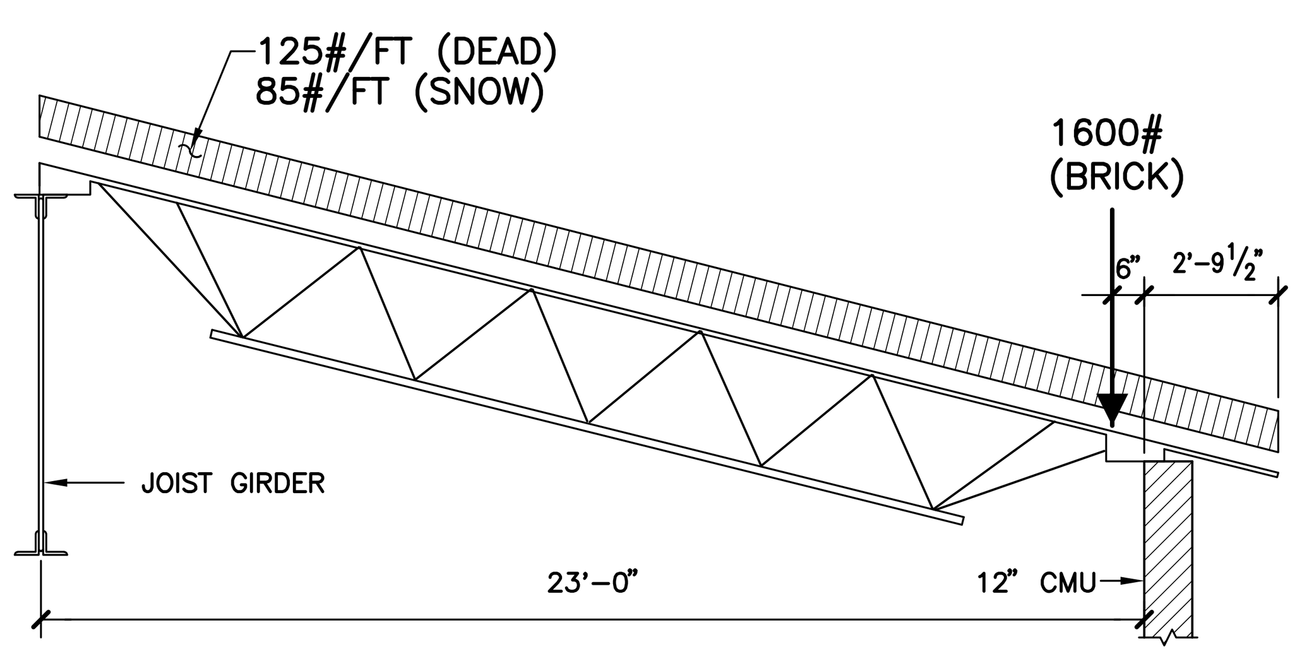
SOBE177A



10KSP1 SPECIAL JOIST DIAGRAM

NOT TO SCALE

SOBE177



16KSP2 SPECIAL JOIST DIAGRAM

SCALE: 1/4" = 1'-0

SOBE177B

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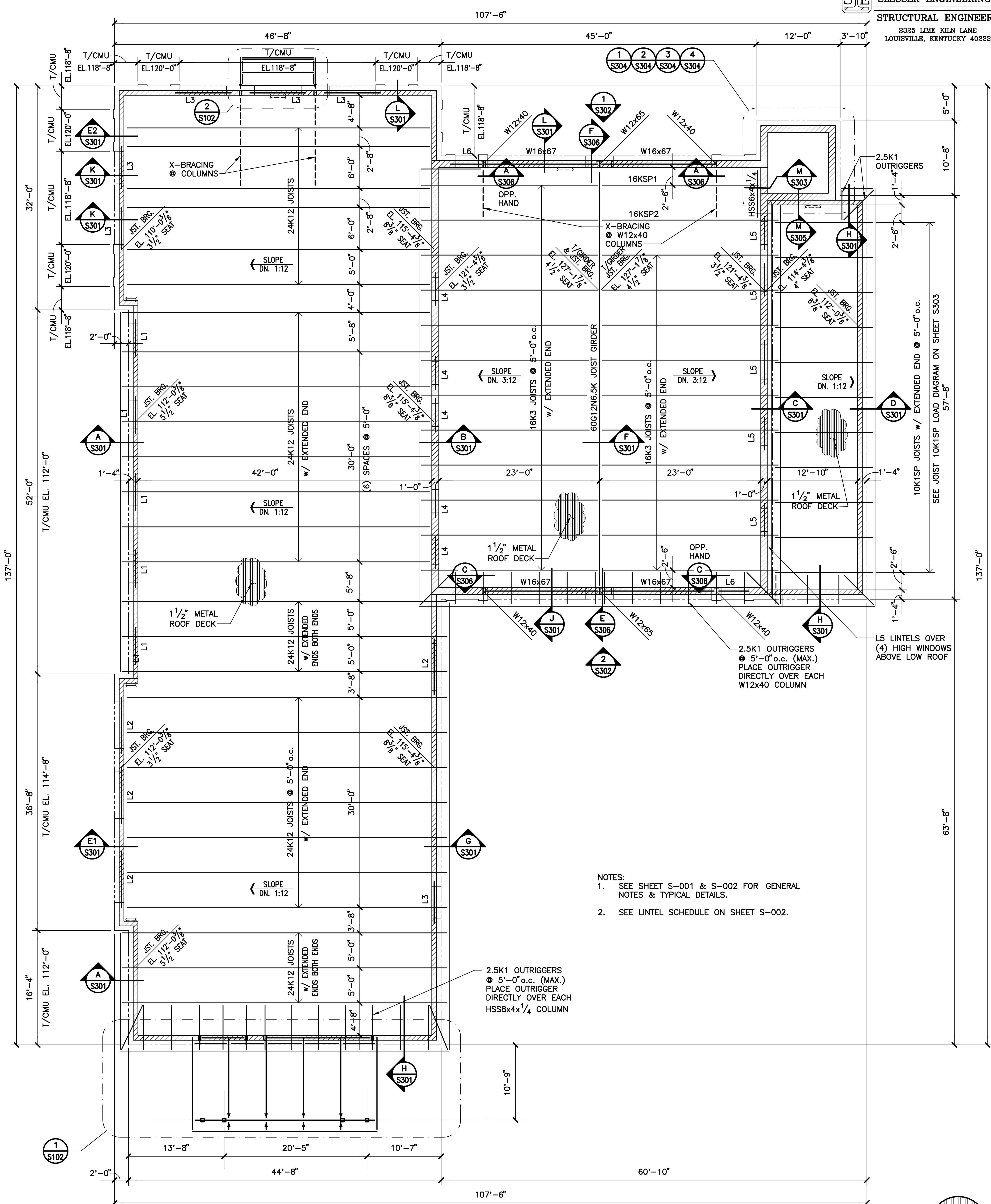

NOTES:
1. SEE SHEET S-001 & S-002 FOR GENERAL NOTES & TYPICAL DETAILS.
2. SEE LINTEL SCHEDULE ON SHEET S-002.

ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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S-102

SLESSER ENGINEERING, INC.
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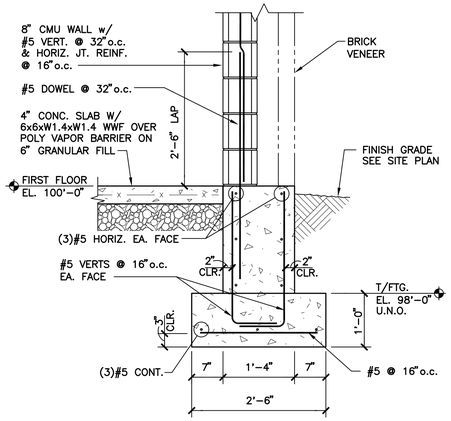

SECTION
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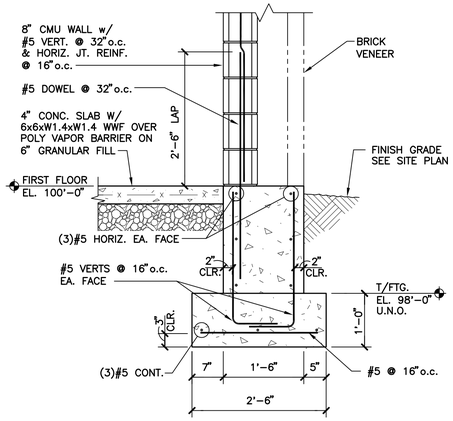
CFES2569 A S201



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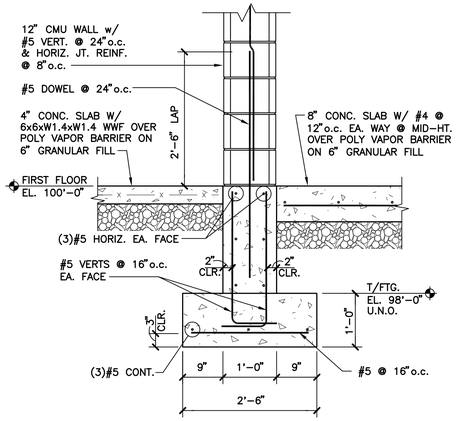

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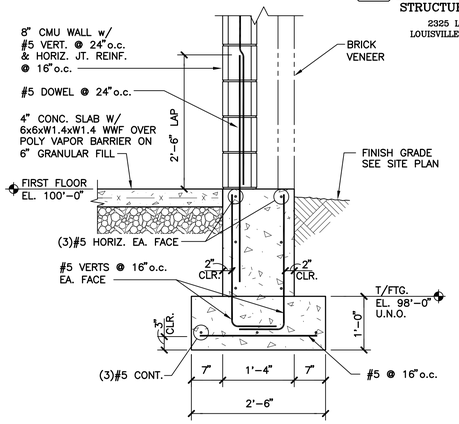

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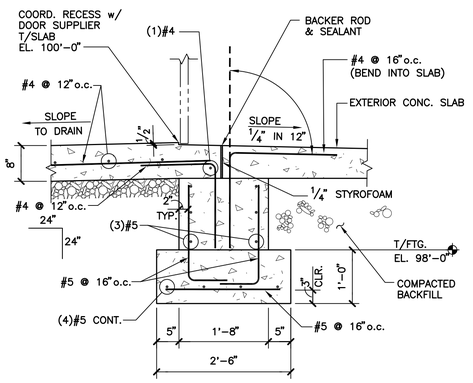

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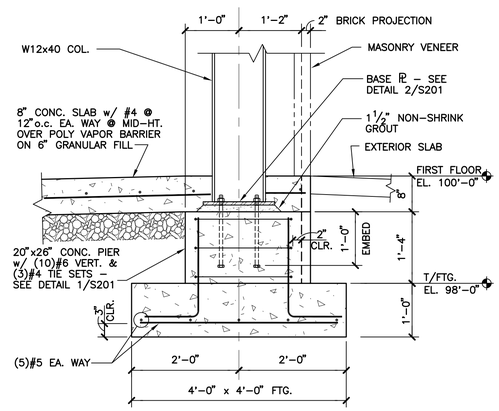

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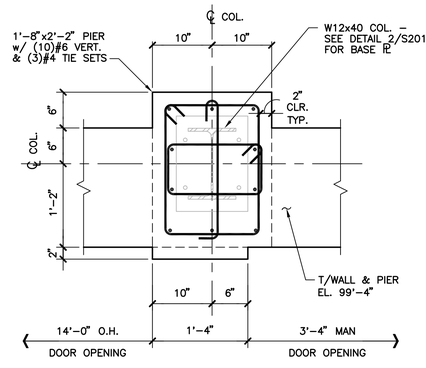
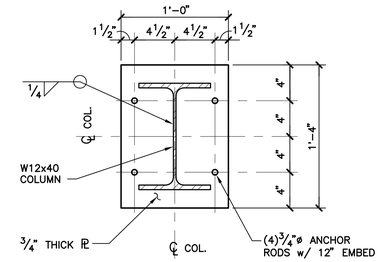
CFES2569D F S201



PIER DETIAL
SCALE: 1" = 1'-0
CFFD131 1 S201



BASE PL DETAIL SBGD520A
SCALE: 1 1/2" = 1'-0
2 S201

NOTE: THIS DRAWING IS INTENDED TO BE PLOTTED IN COLOR. IF THIS SHEET APPEARS IN BLACK AND WHITE, IT IS PLOTTED INCORRECTLY. DISCARD AND OBTAIN AN ACCURATE DRAWING

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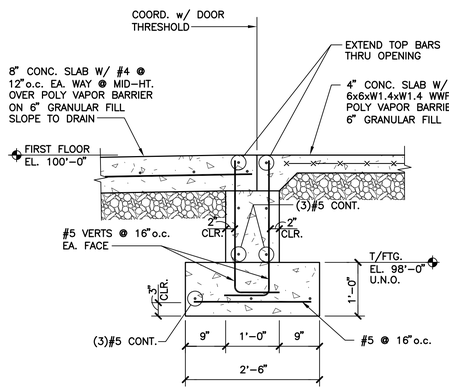
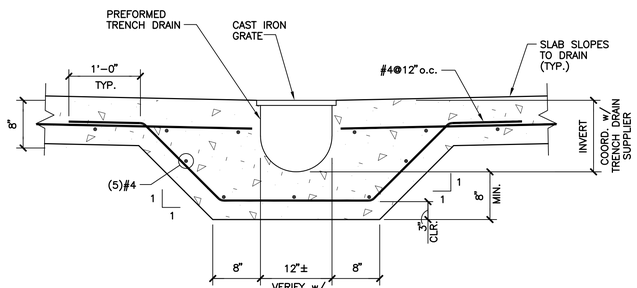


SECTION
SCALE: 3/4" = 1'-0
CFES2569M G S201



SECTION
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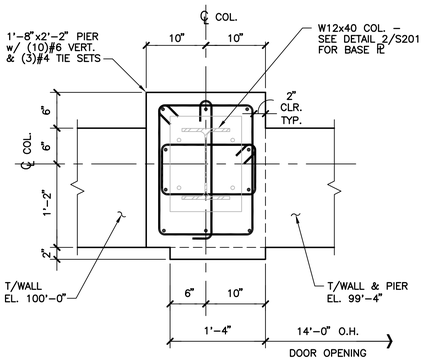
CFFS1037G H S201



PIER DETIAL
SCALE: 1" = 1'-0
CFFD131D 3 S201

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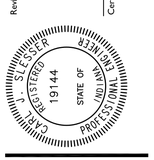
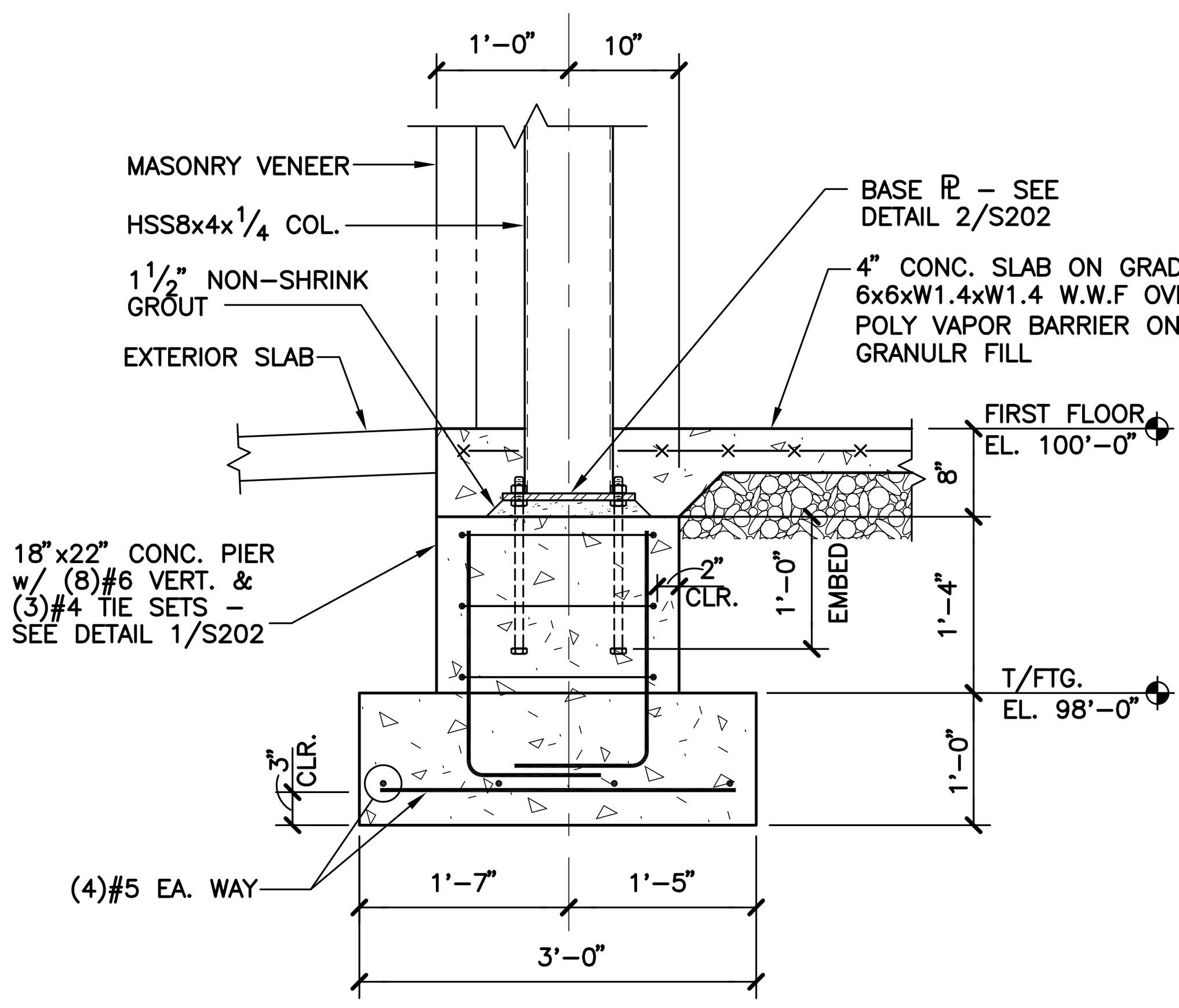


Clarksville Fire Department
Station No. 1
Stansifer Avenue
Clarksville, IN 47129
Town of Clarksville

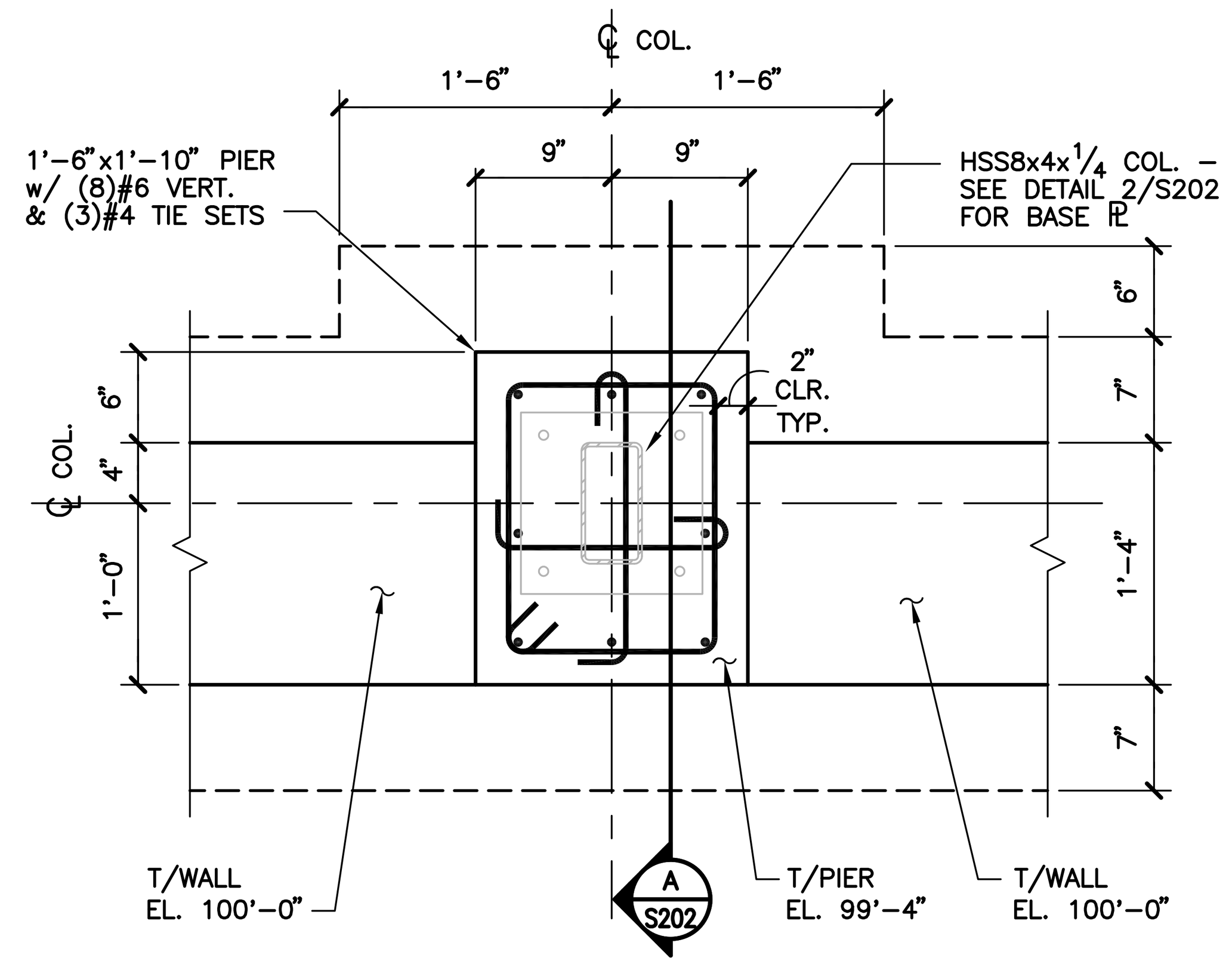
New Facility

Sheet
S-201


1'-0"
10"
MASONRY VENEER
HSS8x4x1/4 COL.
1 1/2" NON-SHRINK GROUT
EXTERIOR SLAB
BASE PL – SEE DETAIL 2/S202
4" CONC. SLAB ON GRADE w/ 6x6xW1.4xW1.4 W.W.F OVER POLY VAPOR BARRIER ON 6" GRANULR FILL
FIRST FLOOR EL. 100'-0"
8"
18"x22" CONC. PIER w/ (8)#6 VERT. & (3)#4 TIE SETS – SEE DETAIL 1/S202
2" CLR.
1'-0"
EMBED
1'-4"
T/FTG. EL. 98'-0"
3" CLR.
1'-0"
(4)#5 EA. WAY
1'-7"
1'-5"
3'-0"

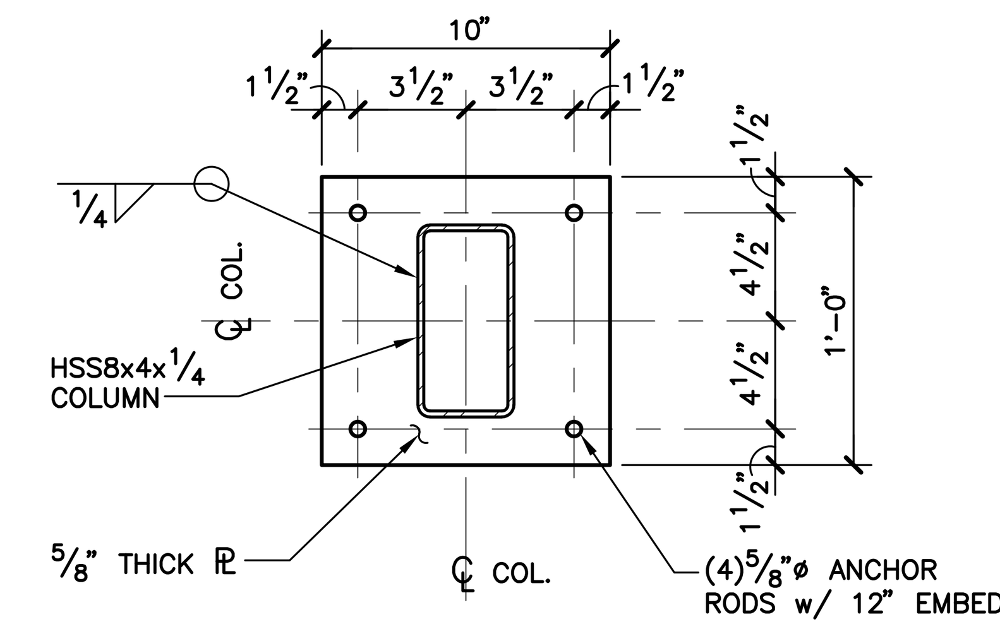
SECTION CFES2569L A/S202

SCALE: 3/4" = 1'-0



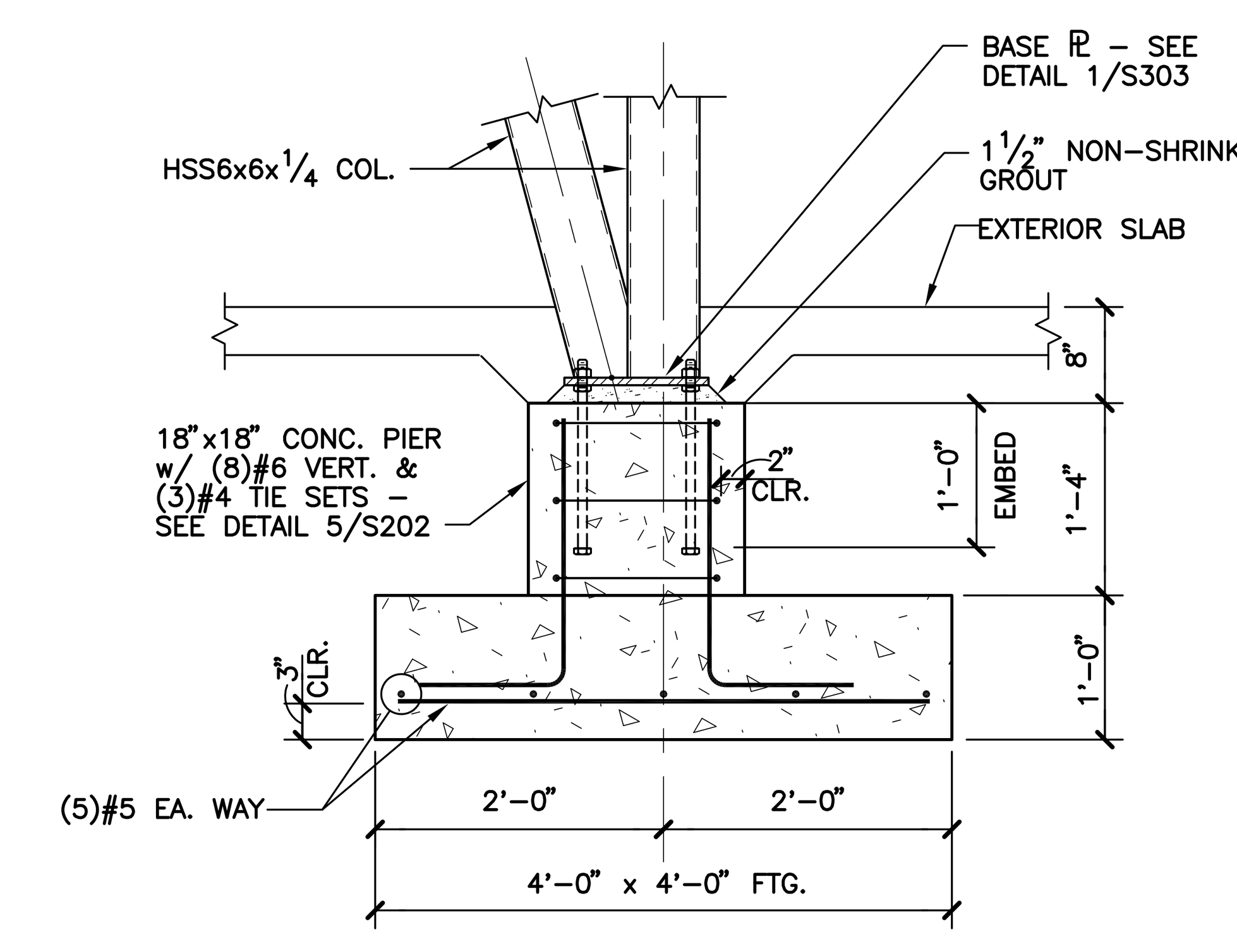
PIER DETIAL CFFD131A 1/S202

SCALE: 1" = 1'-0



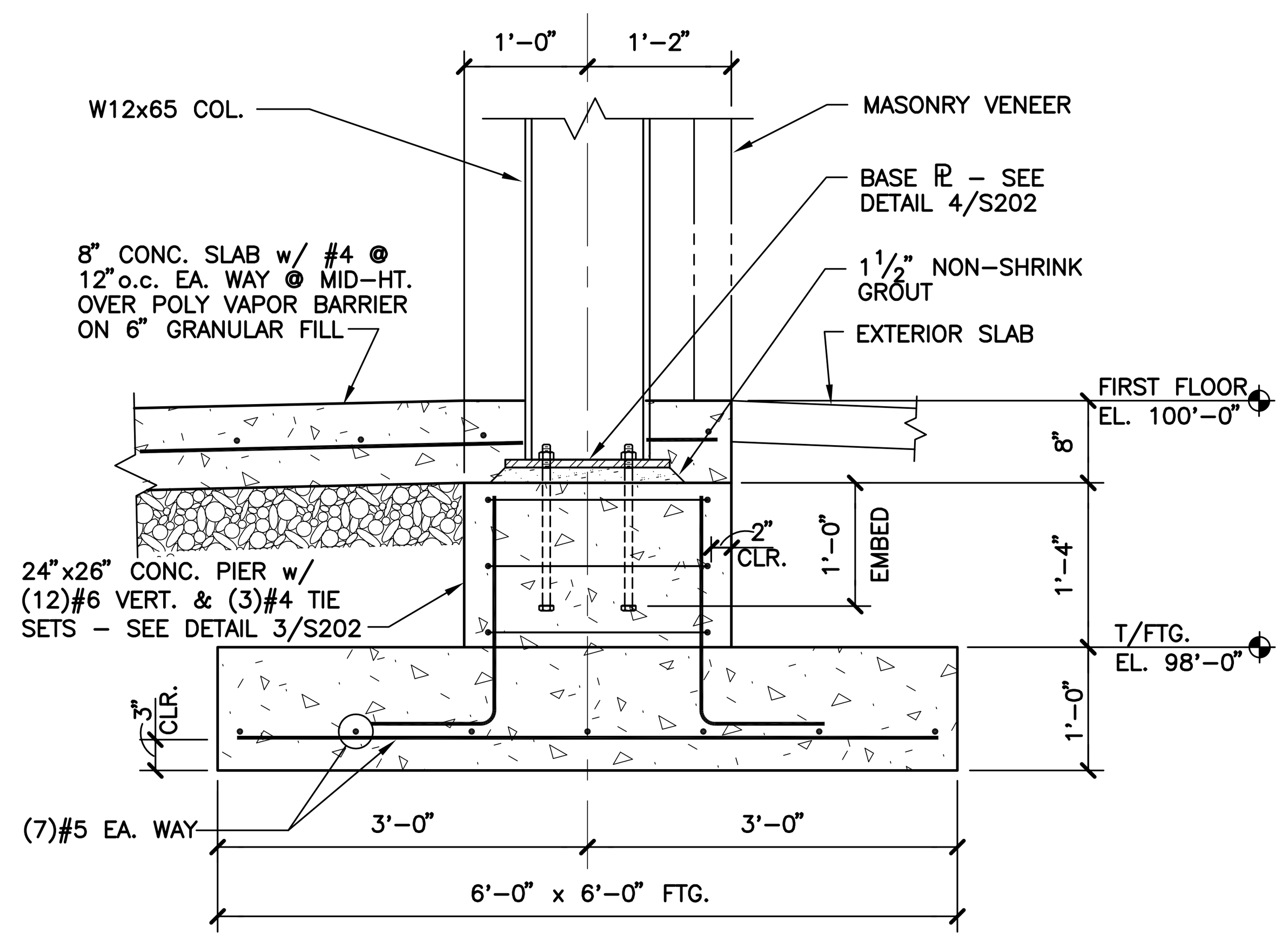
BASE PL DETAIL SBGD520B 2/S202

SCALE: 1 1/2" = 1'-0



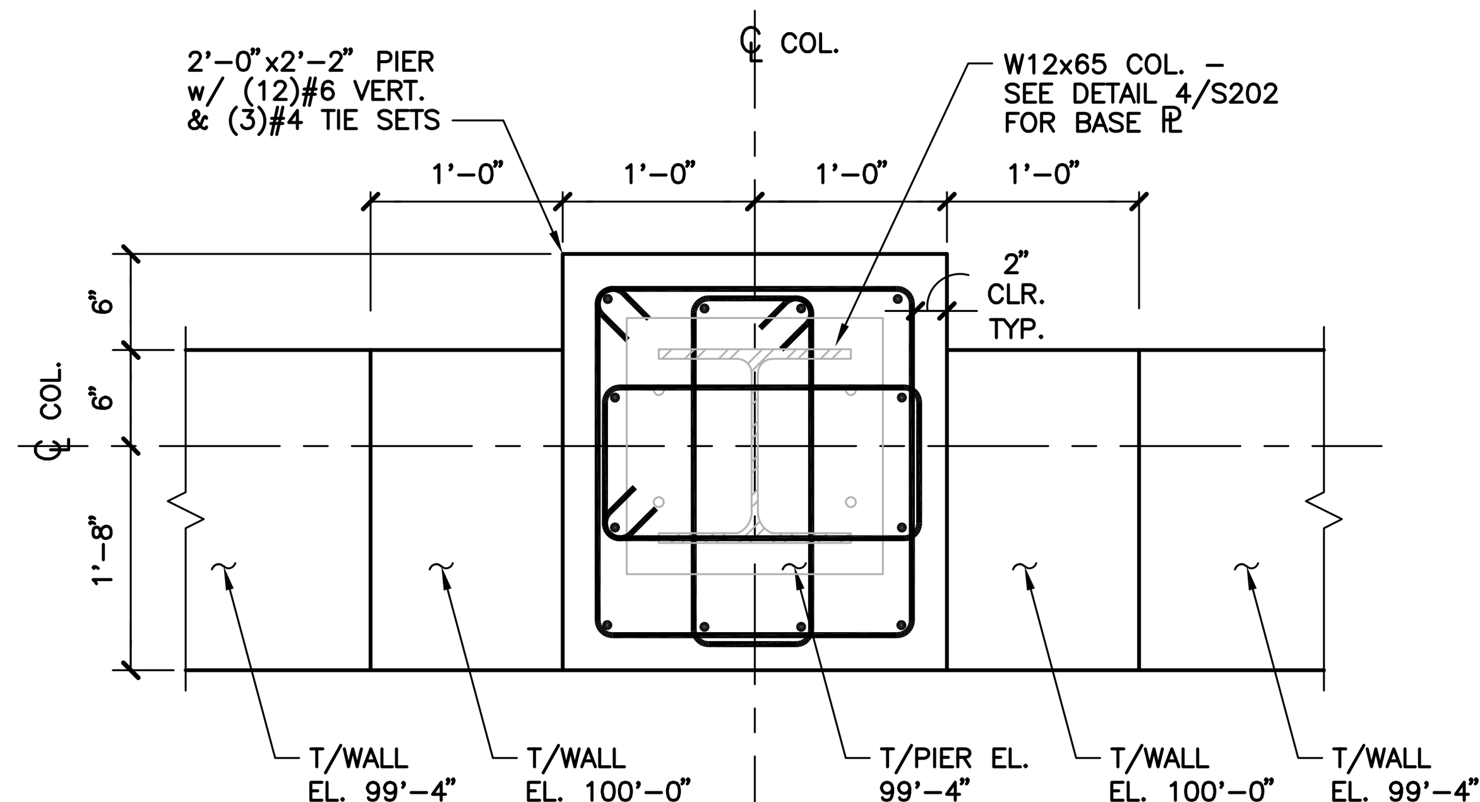
SECTION CFES2569K B/S202

SCALE: 3/4" = 1'-0


1'-0"
1'-2"
W12x65 COL.
MASONRY VENEER
BASE PL – SEE DETAIL 4/S202
8" CONC. SLAB w/ #4 @ 12"o.c. EA. WAY @ MID-HT. OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL
1 1/2" NON-SHRINK GROUT
EXTERIOR SLAB
FIRST FLOOR EL. 100'-0"
8"
2" CLR.
1'-0"
EMBED
1'-4"
24"x26" CONC. PIER w/ (12)#6 VERT. & (3)#4 TIE SETS – SEE DETAIL 3/S202
T/FTG. EL. 98'-0"
3" CLR.
1'-0"
(7)#5 EA. WAY
3'-0"
3'-0"
6'-0" x 6'-0" FTG.

SECTION CFES2569J C/S202

SCALE: 3/4" = 1'-0



PIER DETIAL CFFD131C 3/S202

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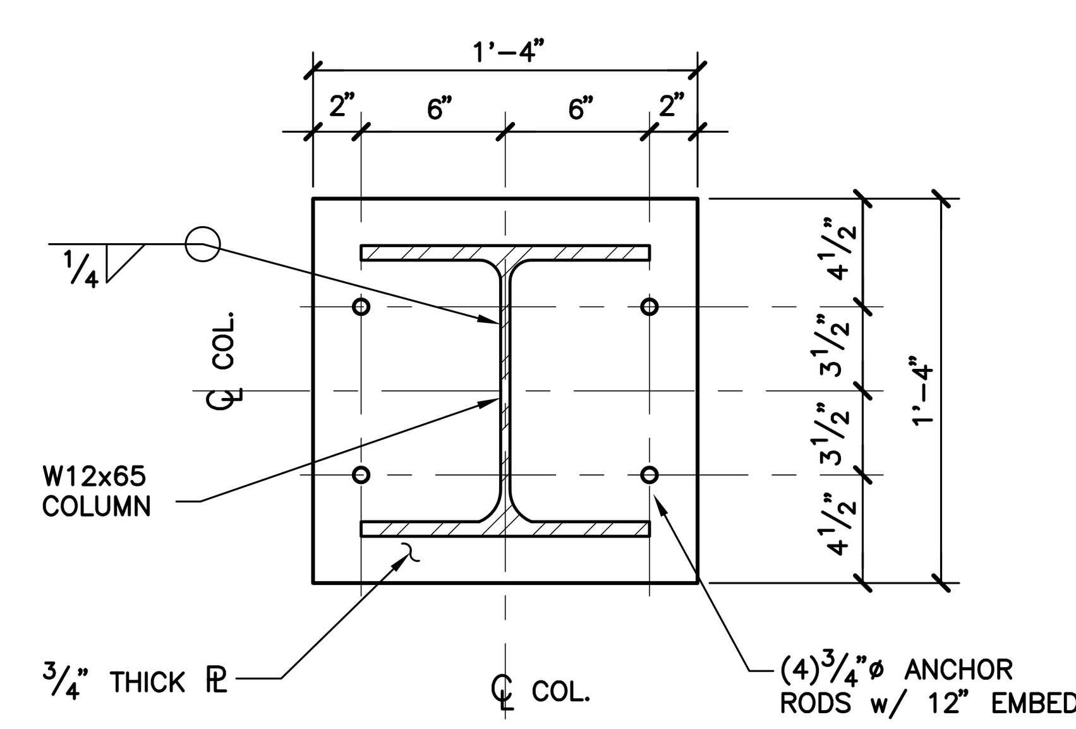

BASE PL DETAIL SBGD520C 4/S202

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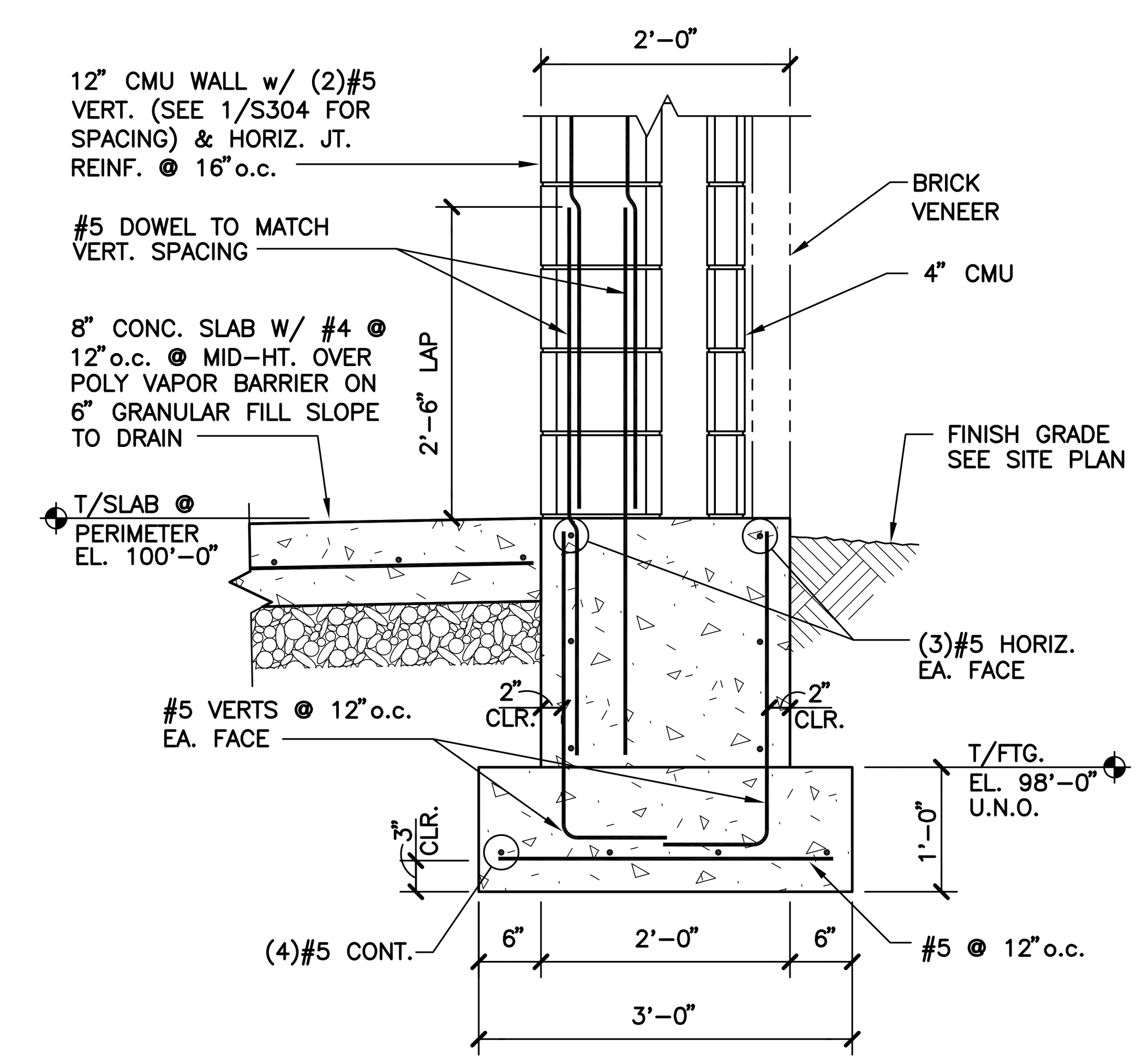
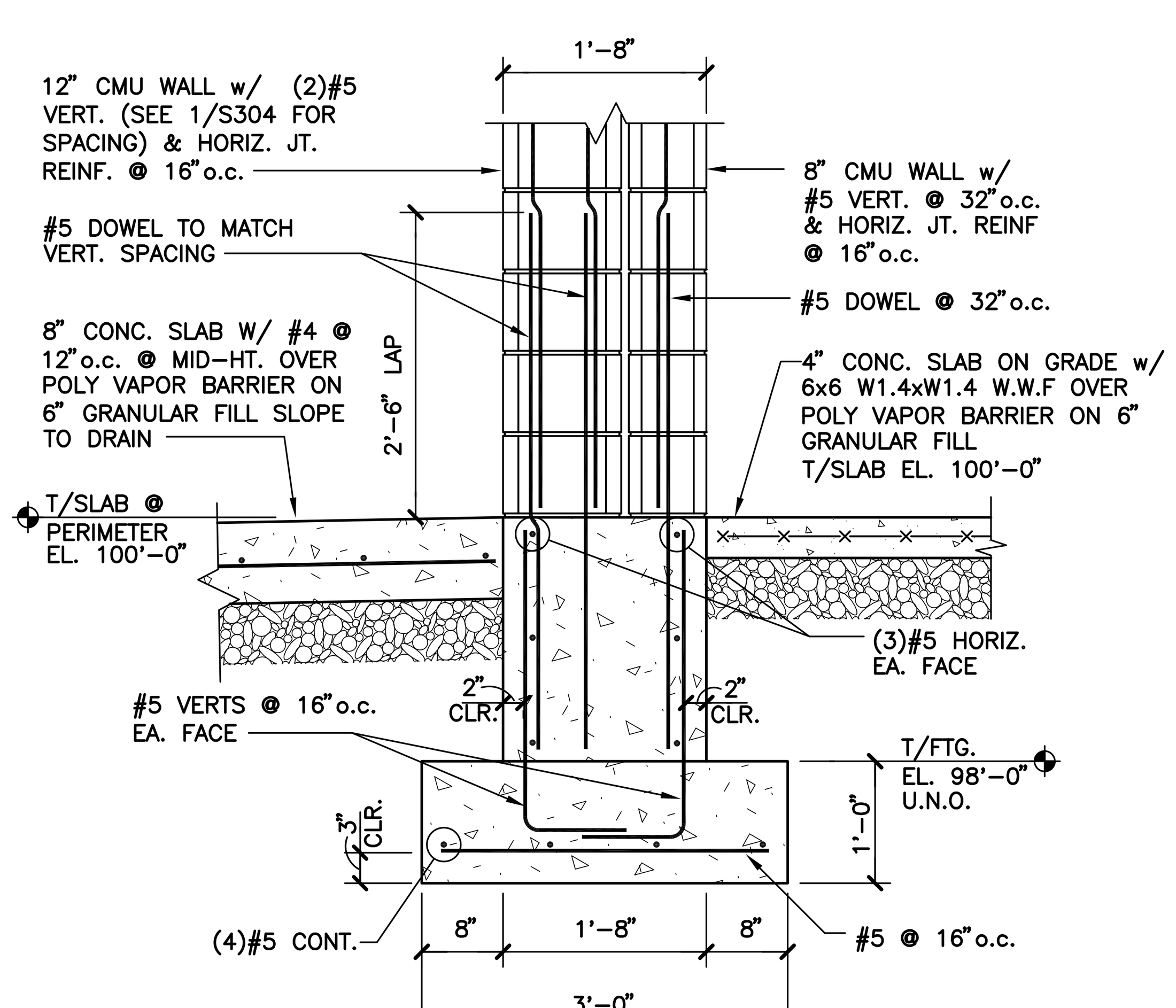

2'-0"
12" CMU WALL w/ (2)#5 VERT. (SEE 1/S304 FOR SPACING) & HORIZ. JT. REINF. @ 16"o.c.
#5 DOWEL TO MATCH VERT. SPACING
BRICK VENEER
4" CMU
8" CONC. SLAB W/ #4 @ 12"o.c. @ MID-HT. OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL SLOPE TO DRAIN
2'-6" LAP
FINISH GRADE SEE SITE PLAN
T/SLAB @ PERIMETER EL. 100'-0"
(3)#5 HORIZ. EA. FACE
#5 VERTS @ 12"o.c. EA. FACE
2" CLR.
2" CLR.
T/FTG. EL. 98'-0" U.N.O.
3" CLR.
1'-0"
(4)#5 CONT.
6"
2'-0"
6"
#5 @ 12"o.c.
3'-0"

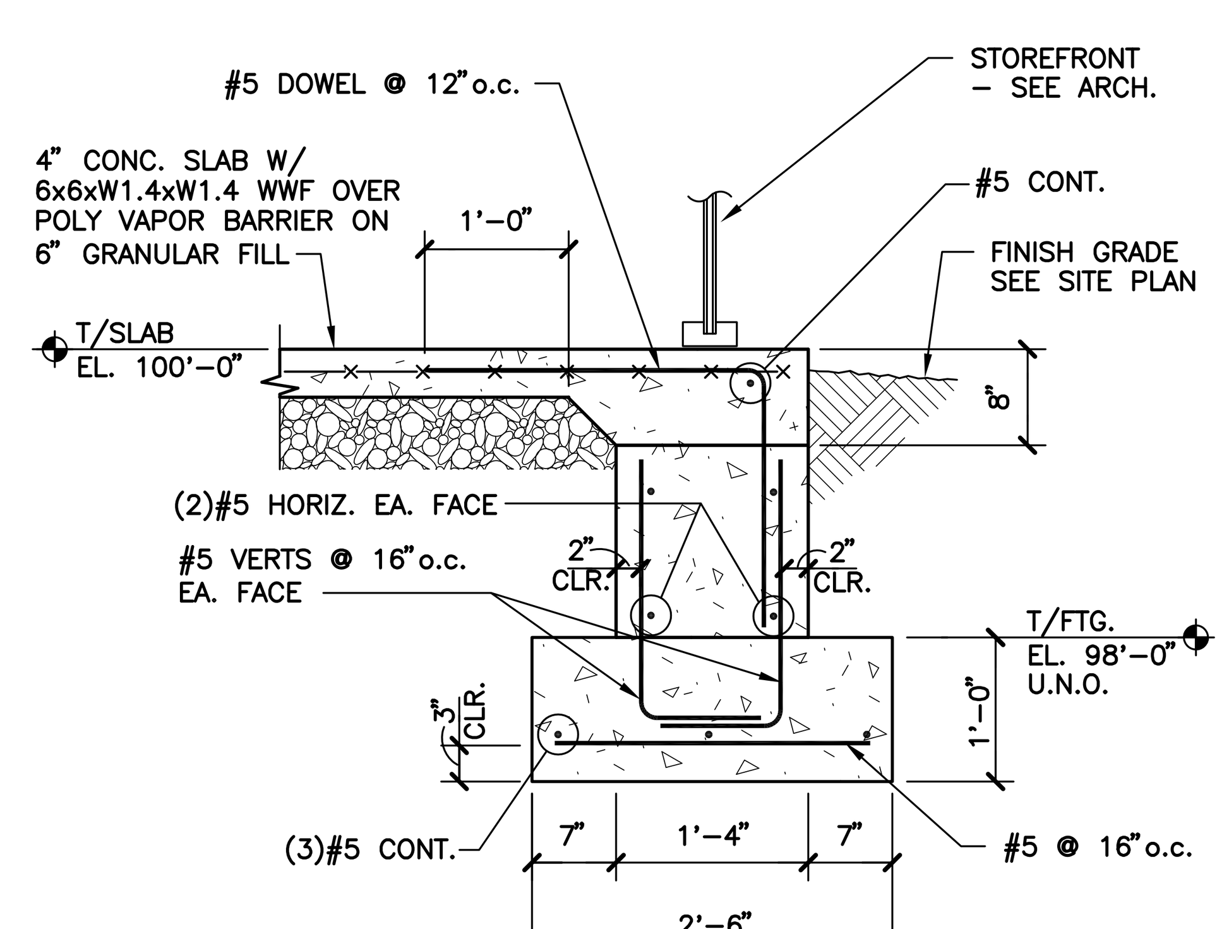
SECTION CFES2569F D/S202

SCALE: 3/4" = 1'-0


1'-8"
12" CMU WALL w/ (2)#5 VERT. (SEE 1/S304 FOR SPACING) & HORIZ. JT. REINF. @ 16"o.c.
8" CMU WALL w/ #5 VERT. @ 32"o.c. & HORIZ. JT. REINF @ 16"o.c.
#5 DOWEL TO MATCH VERT. SPACING
#5 DOWEL @ 32"o.c.
8" CONC. SLAB W/ #4 @ 12"o.c. @ MID-HT. OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL SLOPE TO DRAIN
2'-6" LAP
4" CONC. SLAB ON GRADE w/ 6x6 W1.4xW1.4 W.W.F OVER POLY VAPOR BARRIER ON 6" GRANULAR FILL T/SLAB EL. 100'-0"
T/SLAB @ PERIMETER EL. 100'-0"
(3)#5 HORIZ. EA. FACE
#5 VERTS @ 16"o.c. EA. FACE
2" CLR.
2" CLR.
T/FTG. EL. 98'-0" U.N.O.
3" CLR.
1'-0"
(4)#5 CONT.
8"
1'-8"
8"
#5 @ 16"o.c.
3'-0"

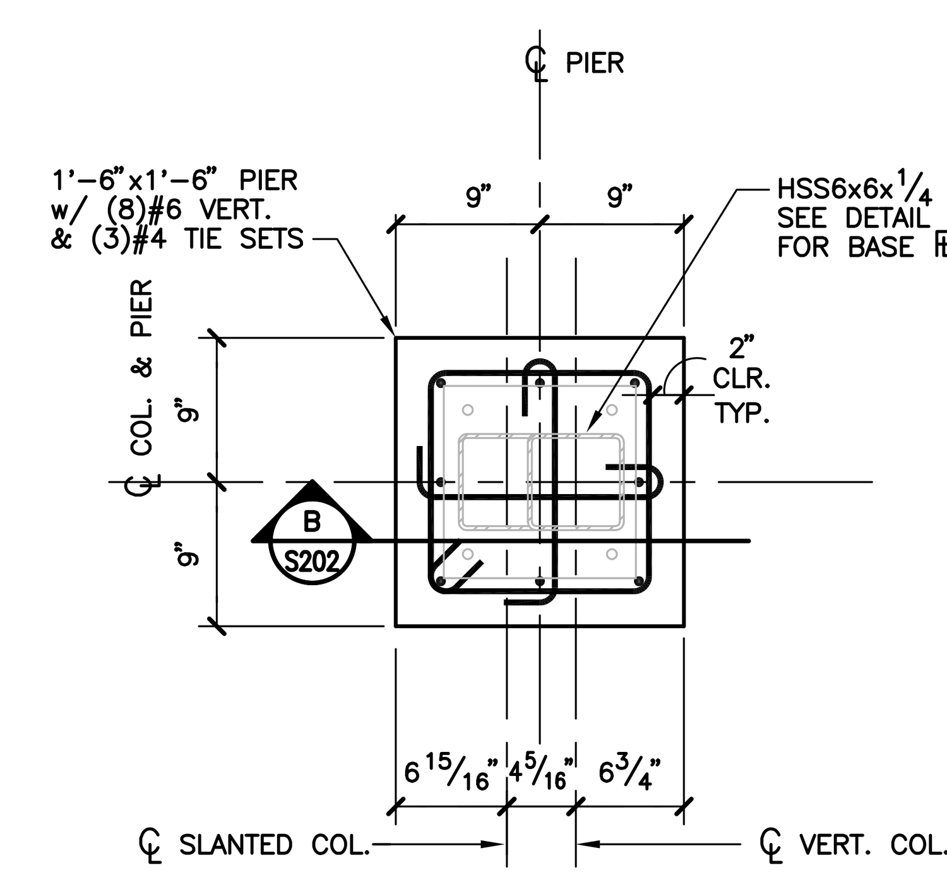
SECTION CFES2569G E/S202

SCALE: 3/4" = 1'-0



SECTION CFES2569H F/S202

SCALE: 3/4" = 1'-0



PIER DETIAL CFFD131E 5/S202

SCALE: 1" = 1'-0

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STRUCTURAL ENGINEERS
2325 LIME KILN LANE
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Drawn	GWM
Checked By	TEL
Project No.	1639.02
Date	11/28/17

Revisions

Certified By [illegible] 11/28/17

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Clarksville Fire Department
Station No. 1

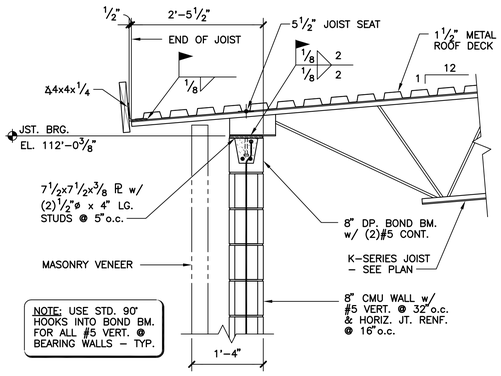
Stansifer Avenue
Clarksville, IN 47129

Town of Clarksville

New Facility

Sheet

S-202



SECTION — MRES350C — A S301

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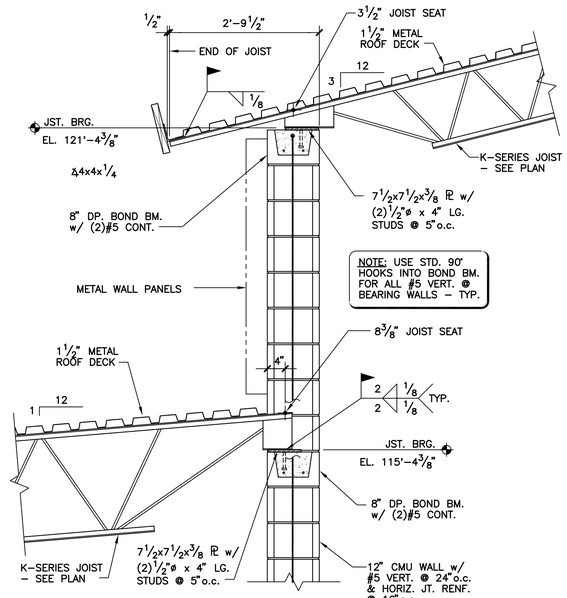

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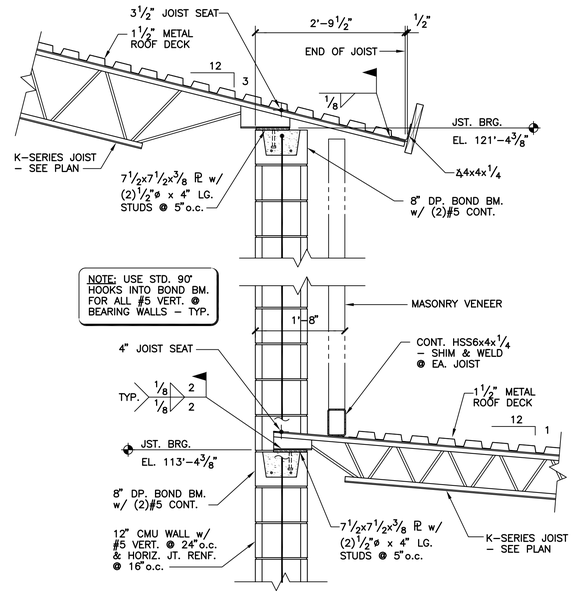

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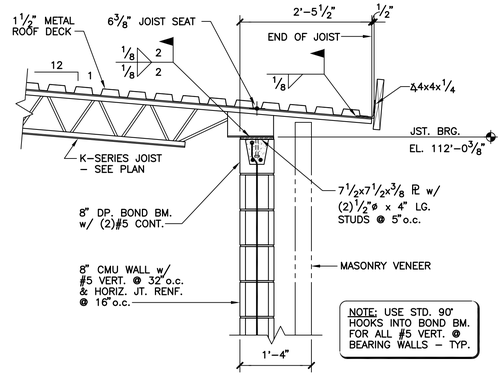


SECTION — MRES350 — D S301

SCALE: 3/4" = 1'-0

SECTION — MRES350D — E1 S301 / E2 S301

SCALE: 3/4" = 1'-0

SECTION — SRES1113 — F S301

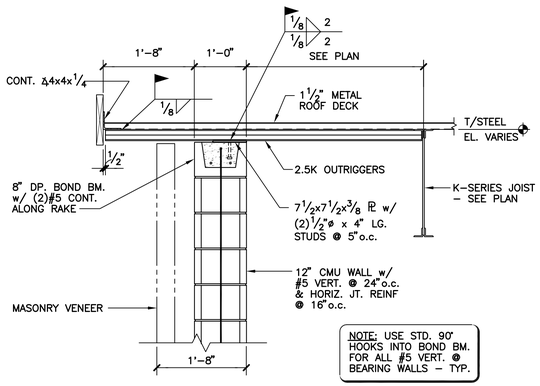
SCALE: 3/4" = 1'-0

SECTION — MRES350G — G S301

SCALE: 3/4" = 1'-0

SECTION — MRES350H — H S301

SCALE: 3/4" = 1'-0



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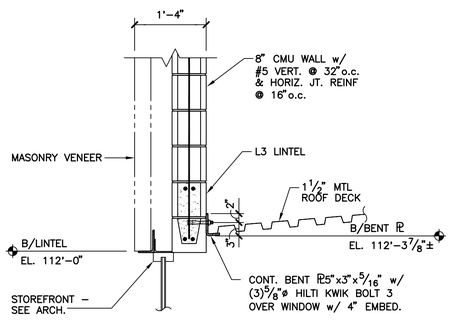

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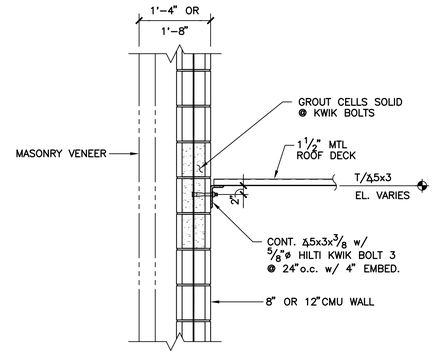

SECTION — MRES350K — L S301

SCALE: 3/4" = 1'-0

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Drawn GWM
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REGISTERED LESSER 19144 STATE OF INDIANA PROFESSIONAL ENGINEER

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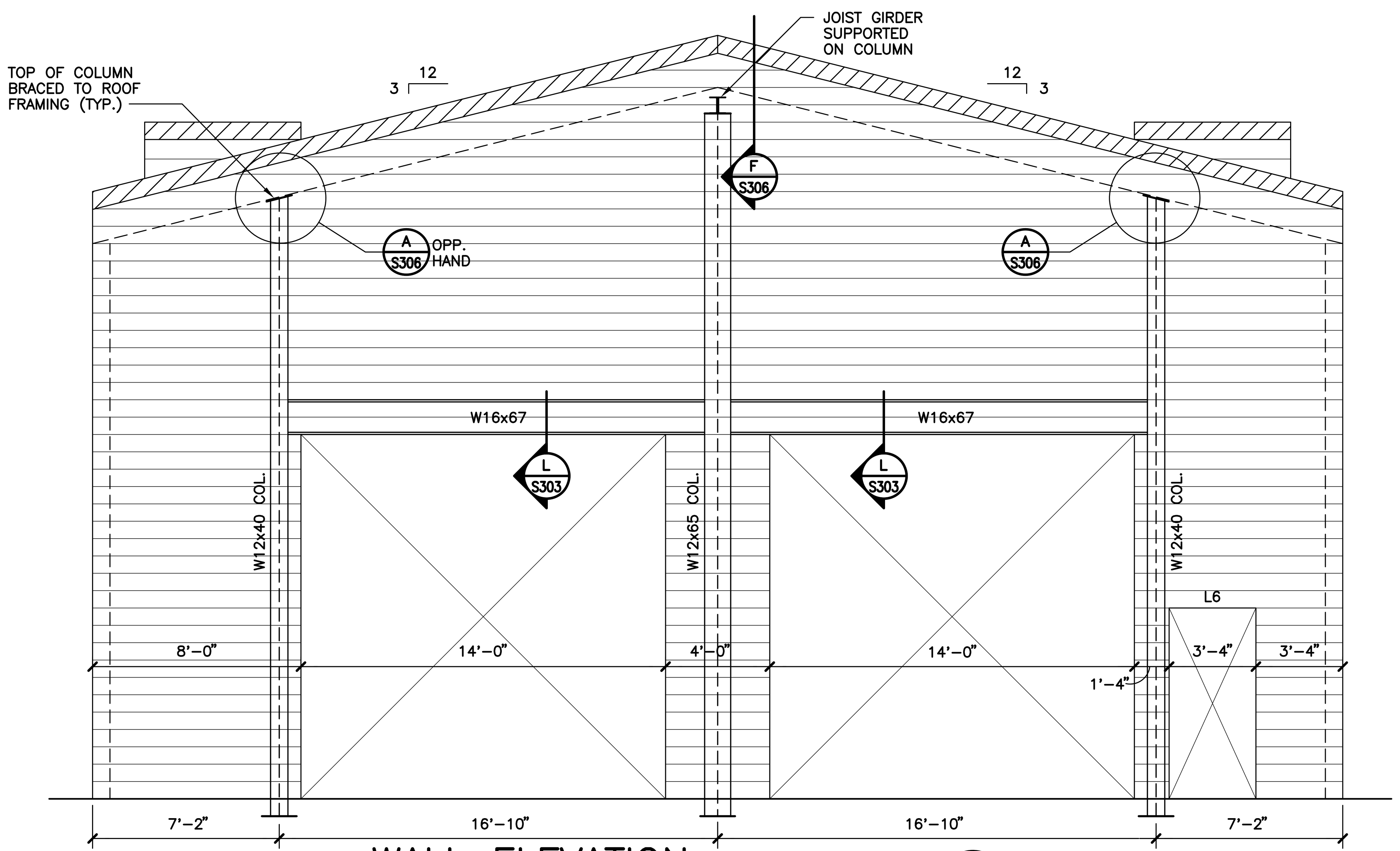
Town of Clarksville

New Facility

Sheet

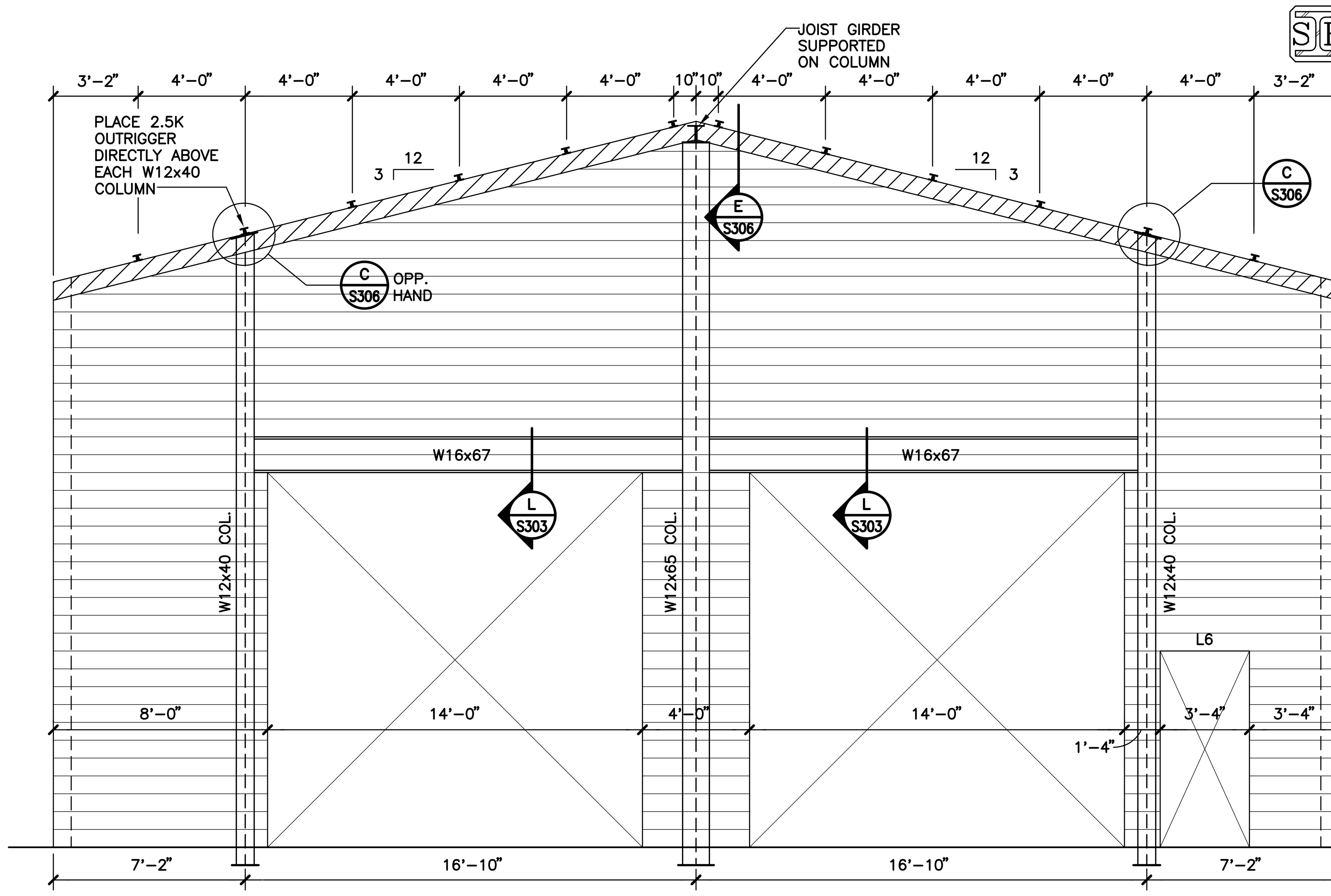
S-301



WALL ELEVATION

MOBE015A 1 S302

SCALE: 1/4" = 1'–0



WALL ELEVATION

MOBE015B 2 S302

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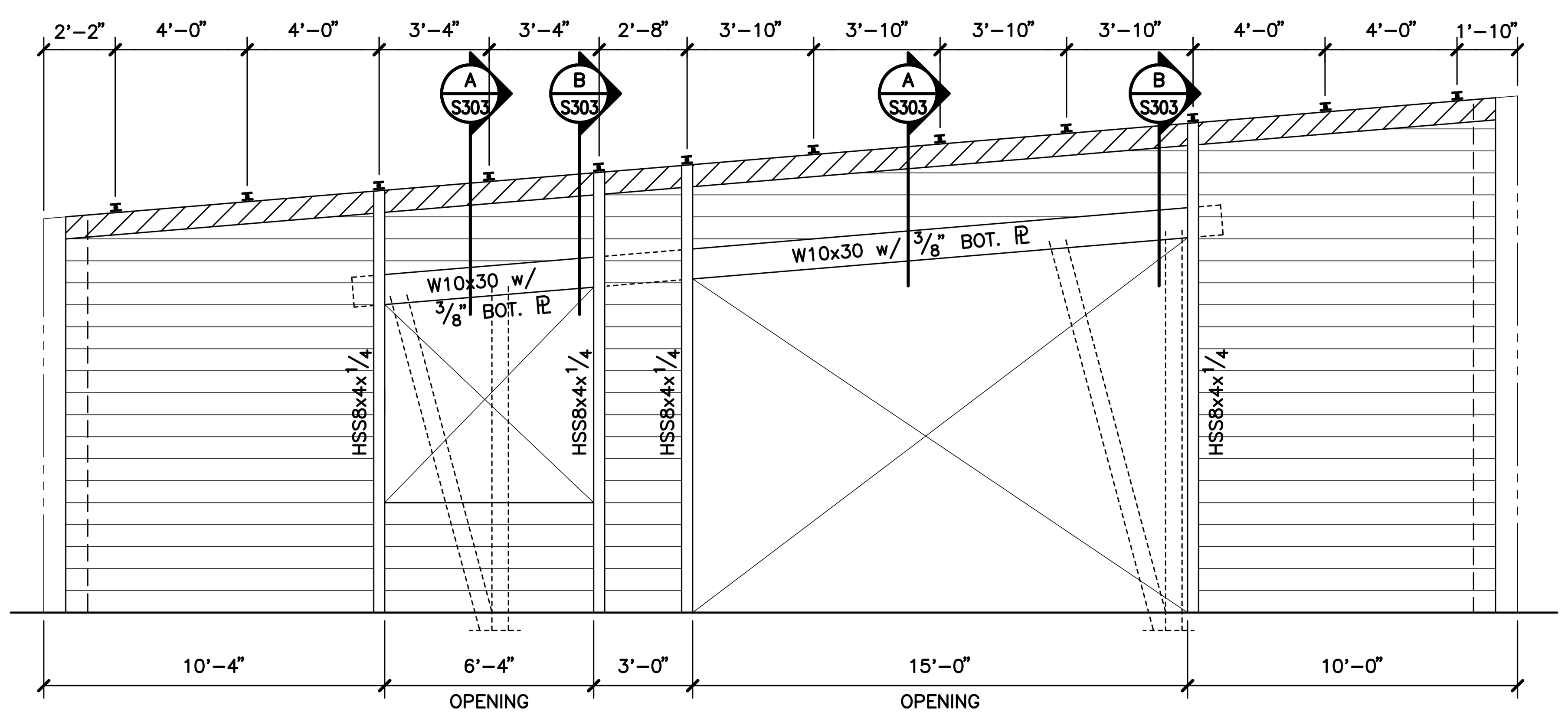

WALL ELEVATION

MOBE015 3 S302

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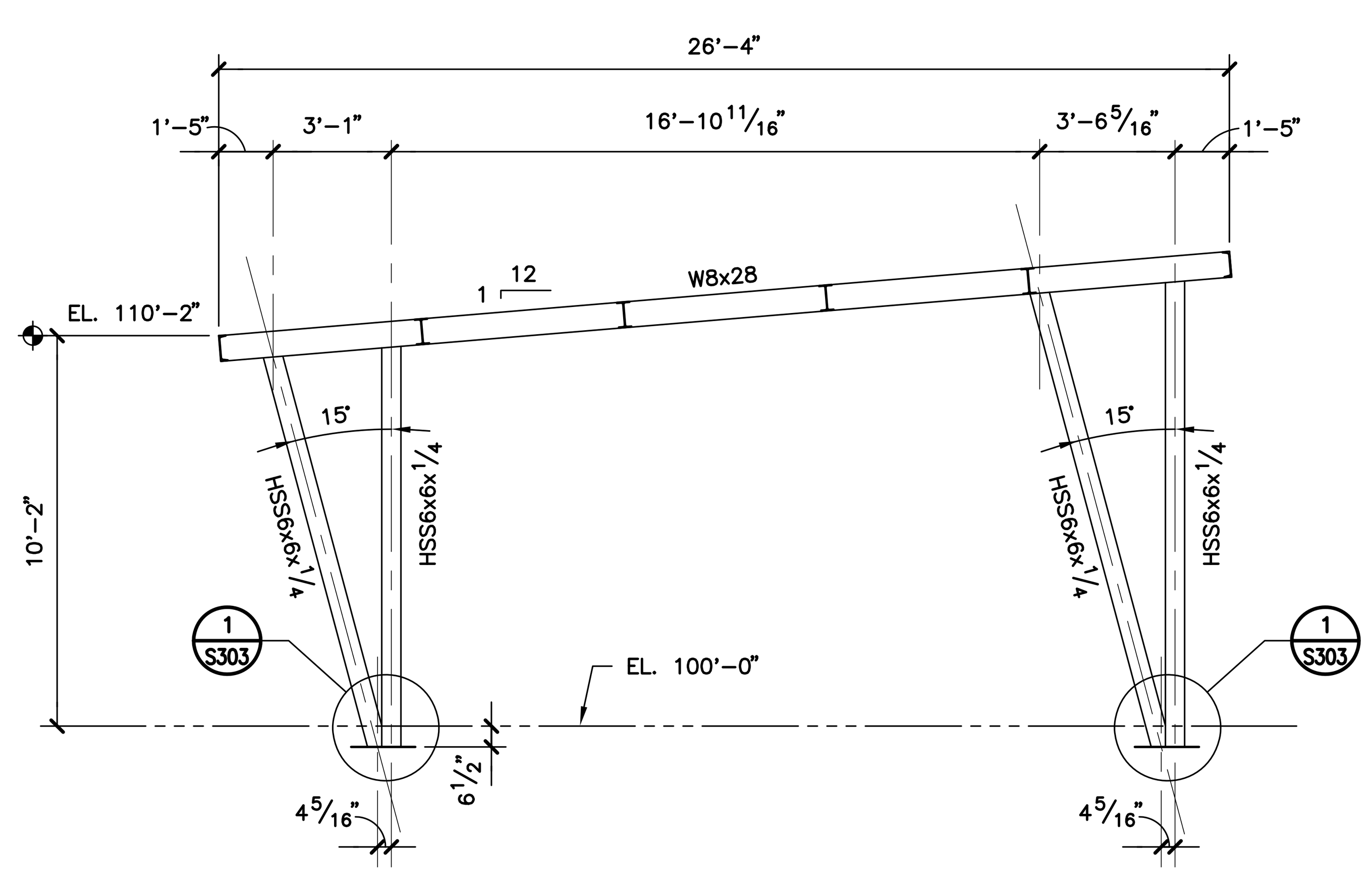

FRAME ELEVATION

SRBS010 4 S302

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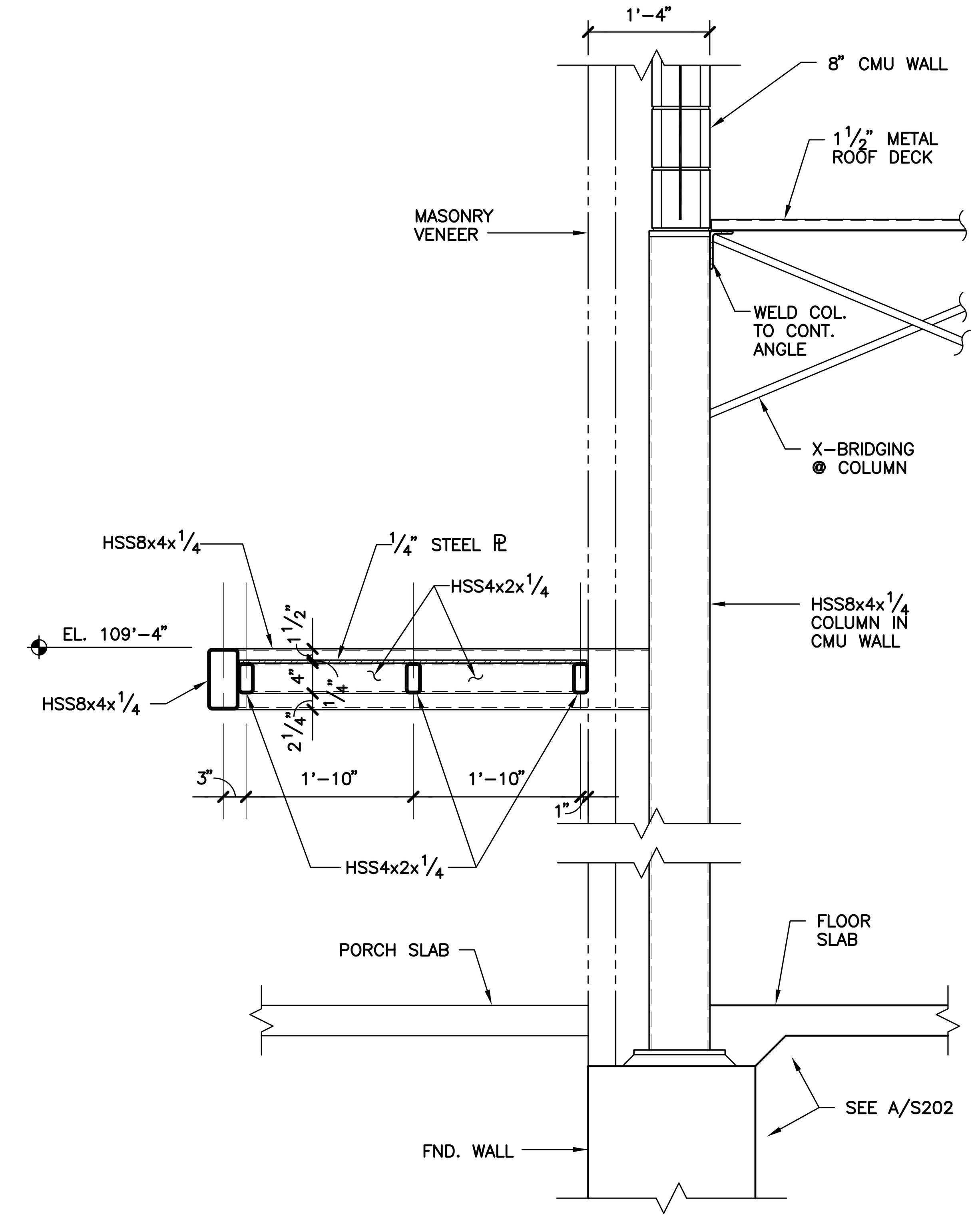

SECTION

SRES1113F A S302

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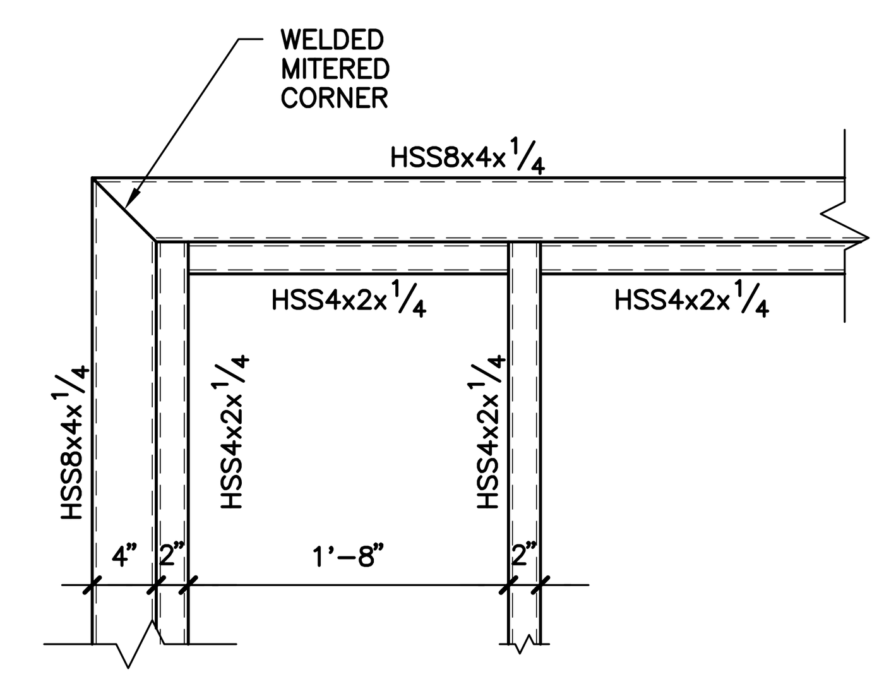

PLAN DETAIL

SRFD038 5 S302

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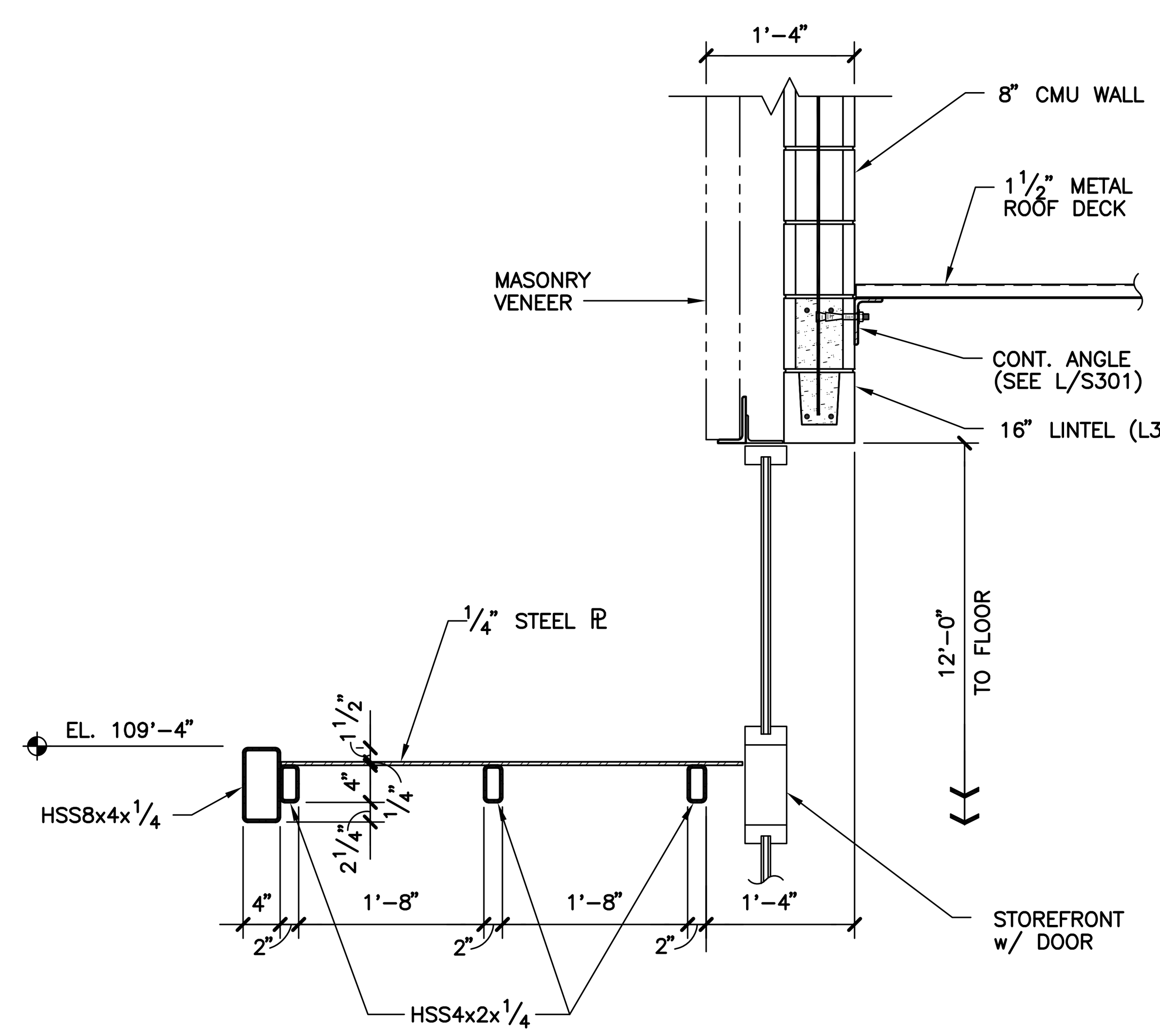

SECTION

SRES1113G B S302

SCALE: 3/4" = 1'–0

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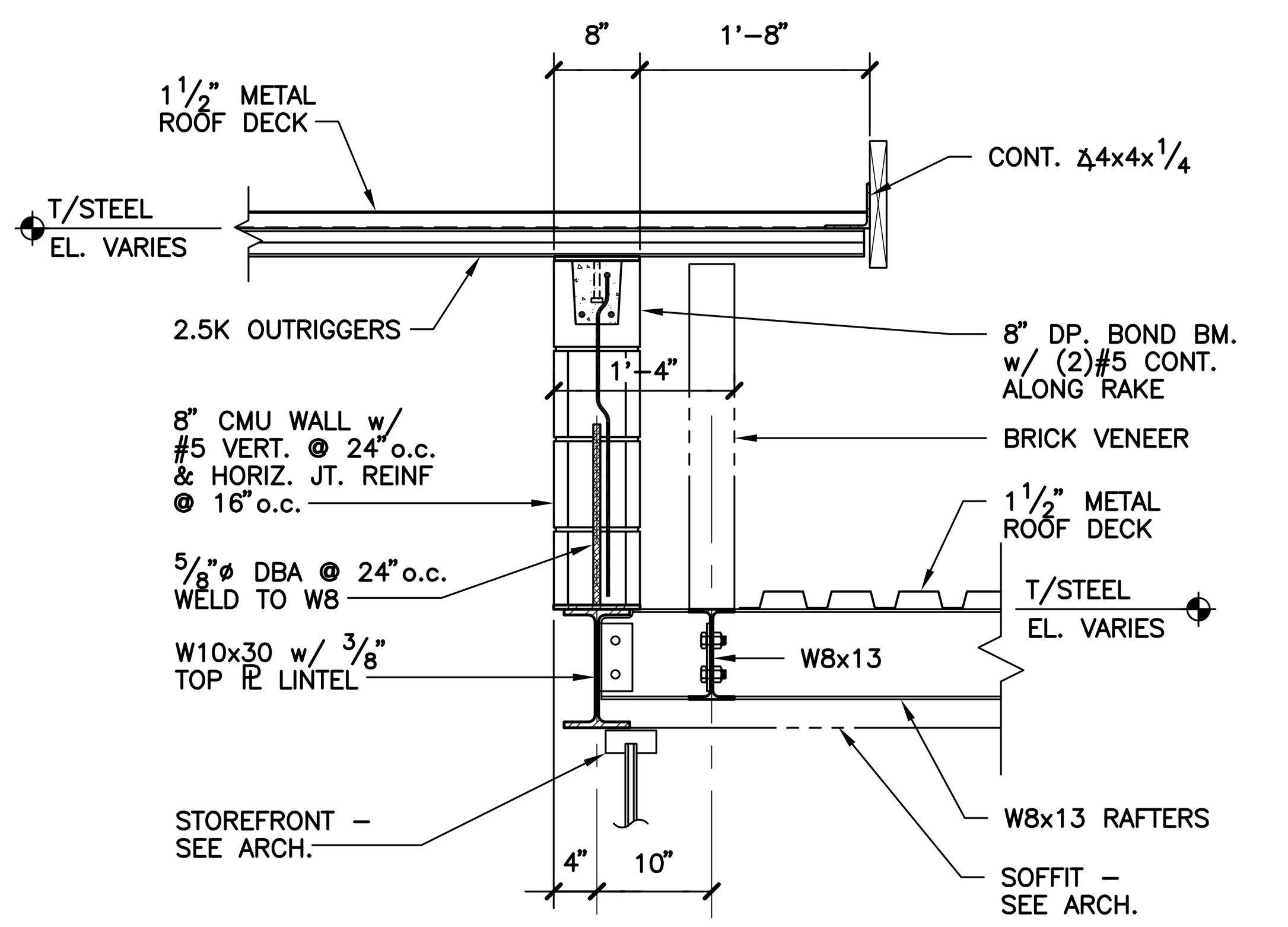
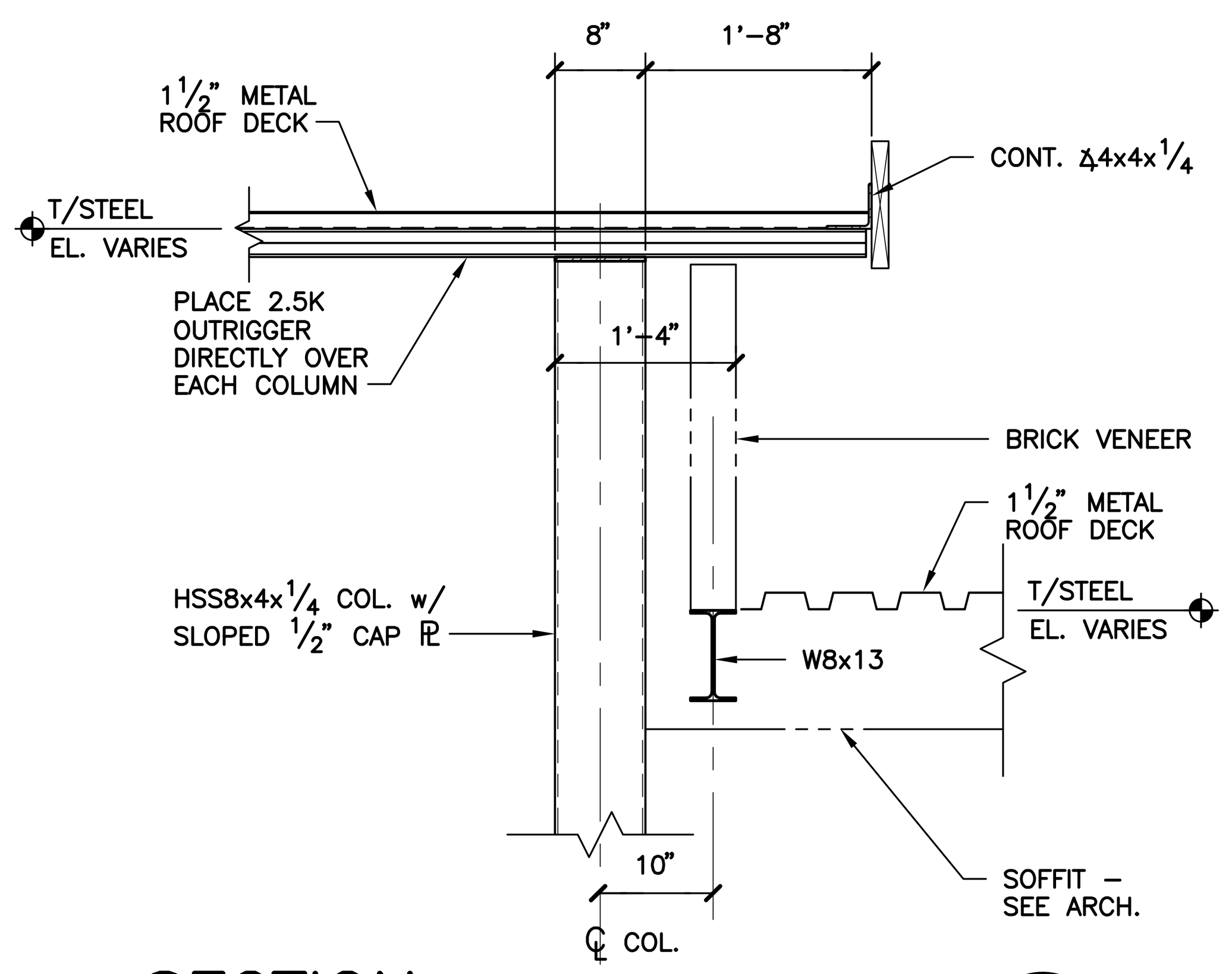

8"
1'–8"
1 1/2" METAL ROOF DECK
CONT. ∠4x4x1/4
T/STEEL
EL. VARIES
2.5K OUTRIGGERS
8" DP. BOND BM. w/ (2)#5 CONT. ALONG RAKE
1'–4"
8" CMU WALL w/ #5 VERT. @ 24"o.c. & HORIZ. JT. REINF @ 16"o.c.
BRICK VENEER
1 1/2" METAL ROOF DECK
5/8"ø DBA @ 24"o.c. WELD TO W8
T/STEEL
EL. VARIES
W10x30 w/ 3/8" TOP ℙ LINTEL
W8x13
STOREFRONT – SEE ARCH.
W8x13 RAFTERS
4"
10"
SOFFIT – SEE ARCH.

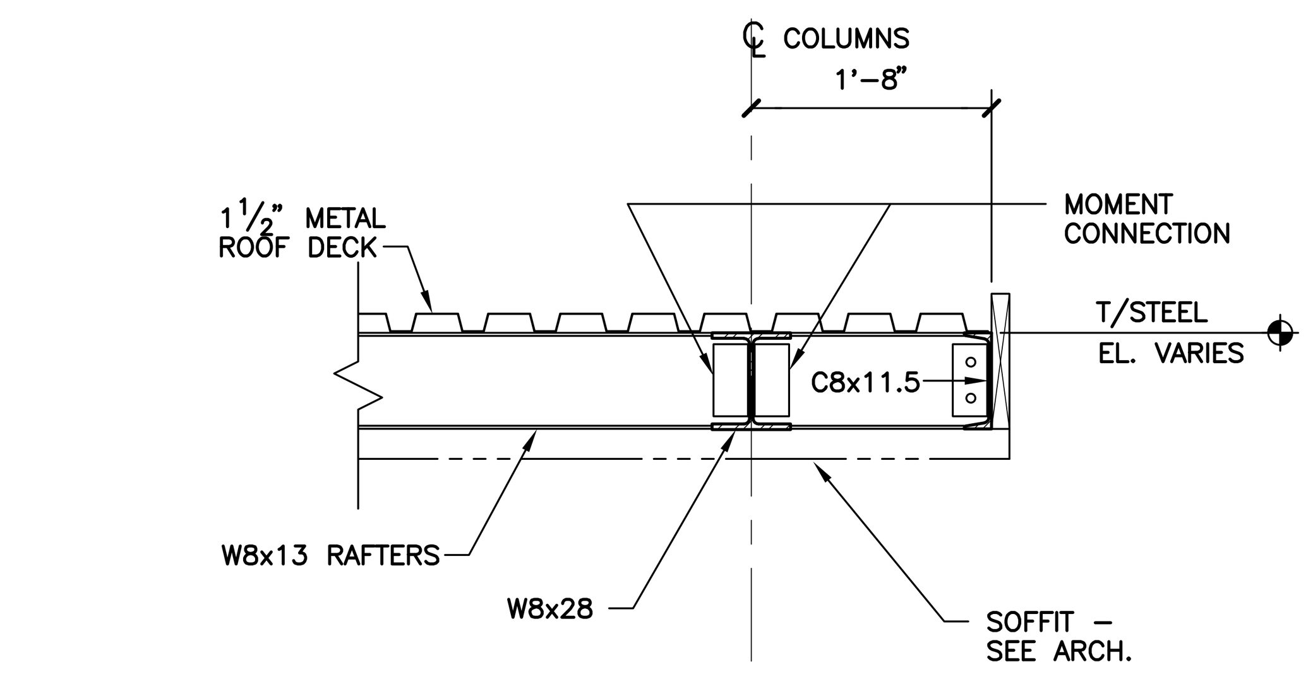
SECTION (A / S303) MRES350E

SCALE: 3/4" = 1'–0



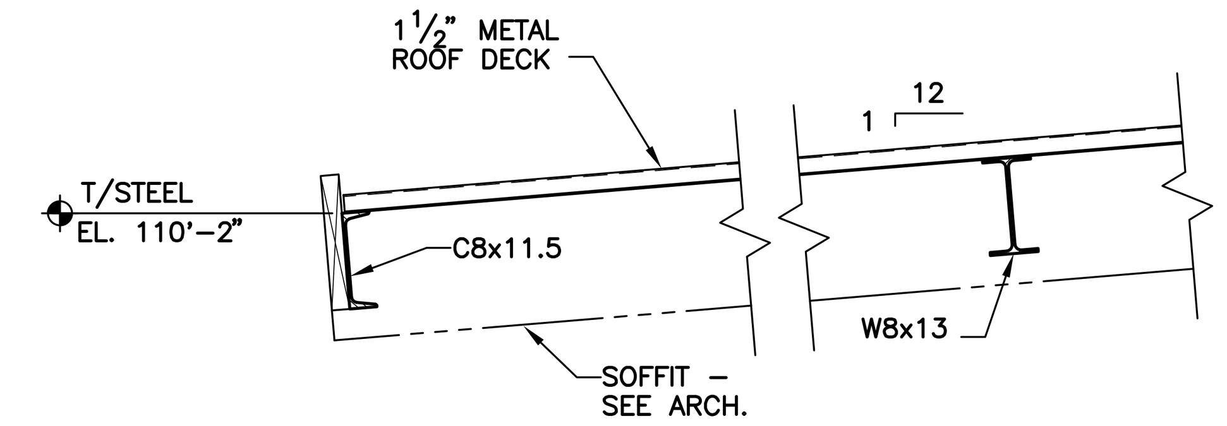
SECTION (B / S303) MRES350F

SCALE: 3/4" = 1'–0



SECTION (C / S303) SRES1113A

SCALE: 3/4" = 1'–0



SECTION (D / S303) SRES1113B

SCALE: 3/4" = 1'–0

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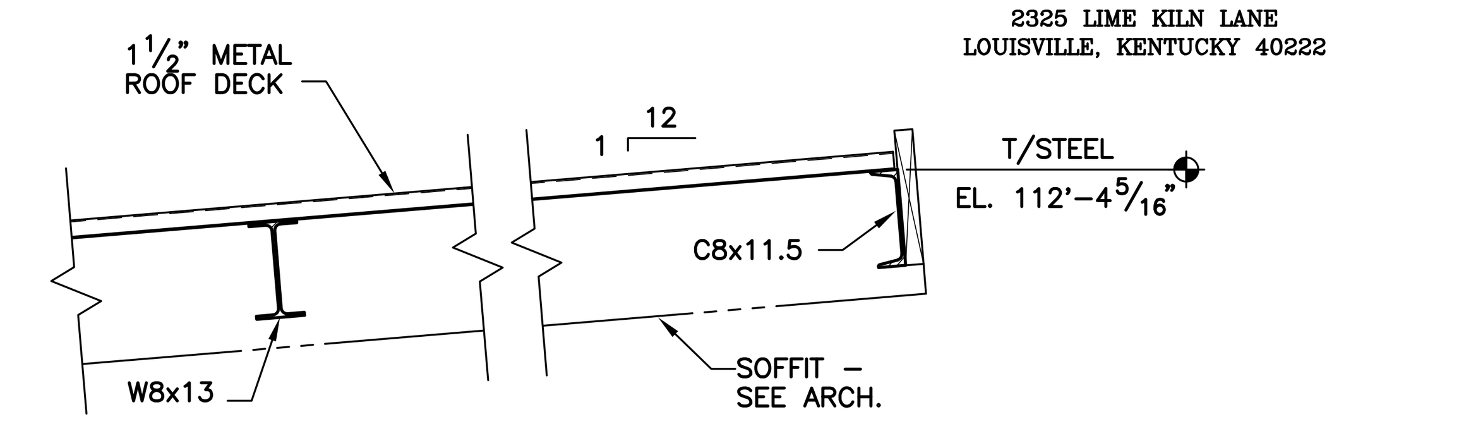
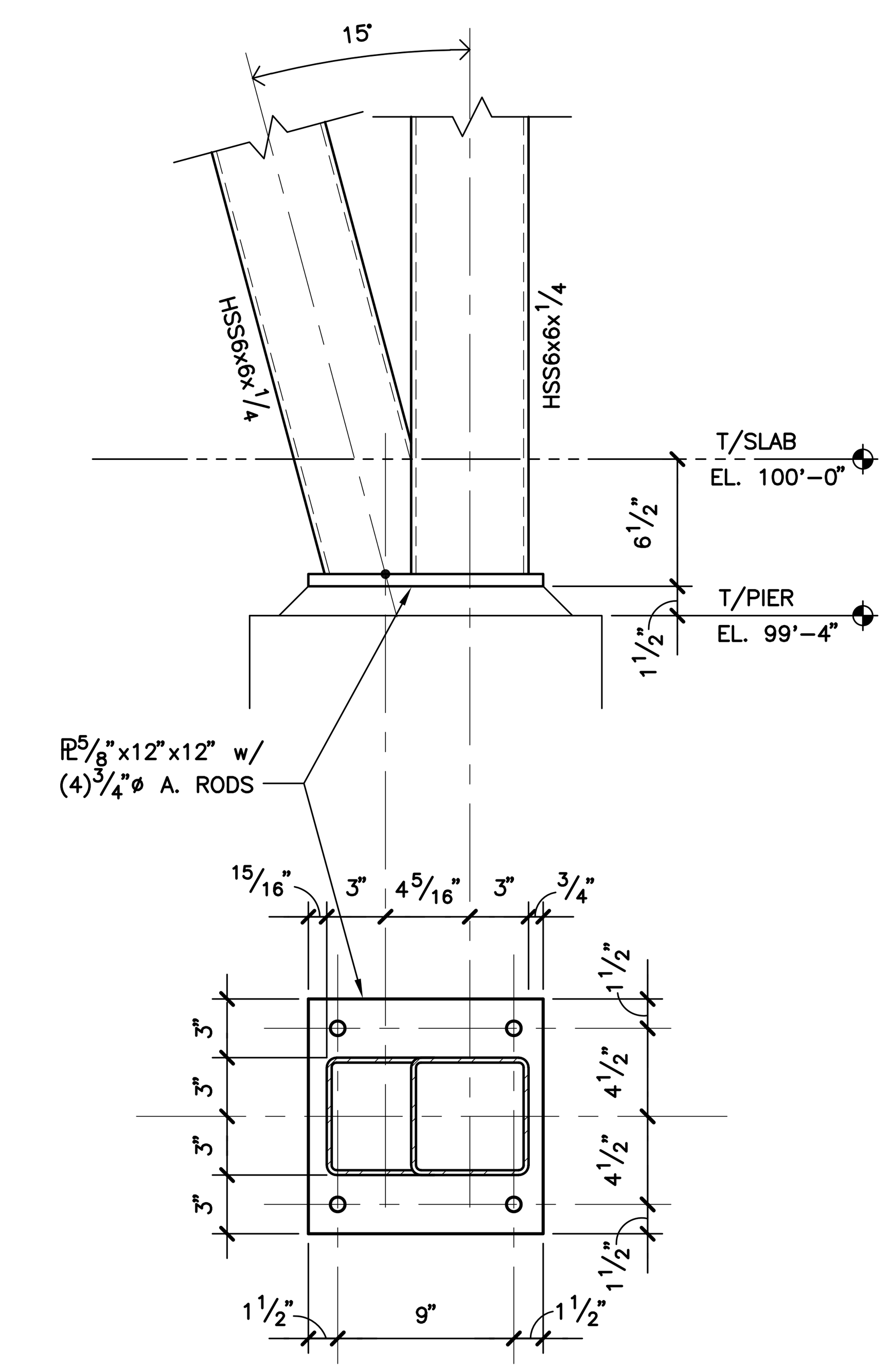
LOUISVILLE, KENTUCKY 40222



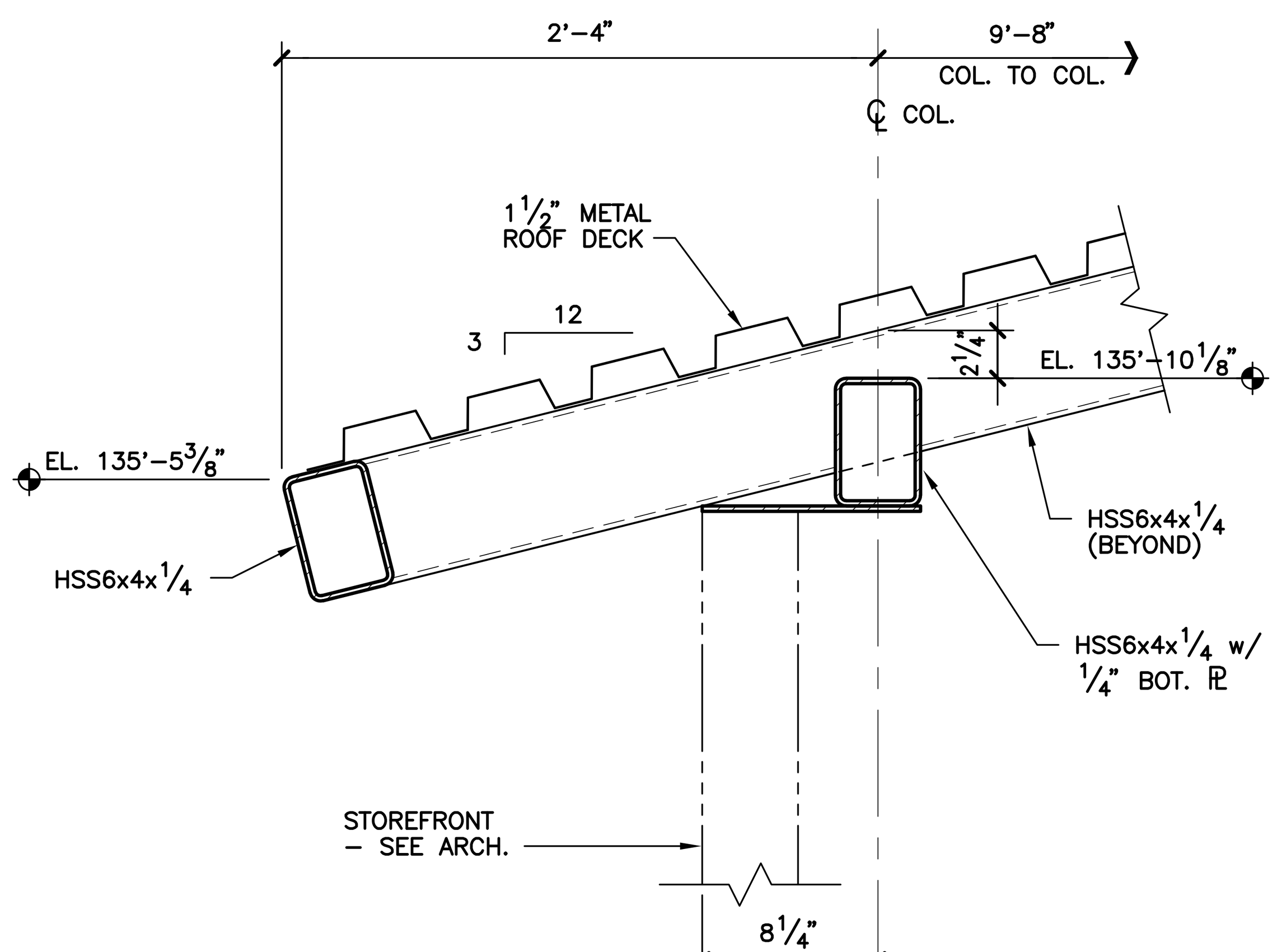
SECTION (E / S303) SRES1113C

SCALE: 3/4" = 1'–0



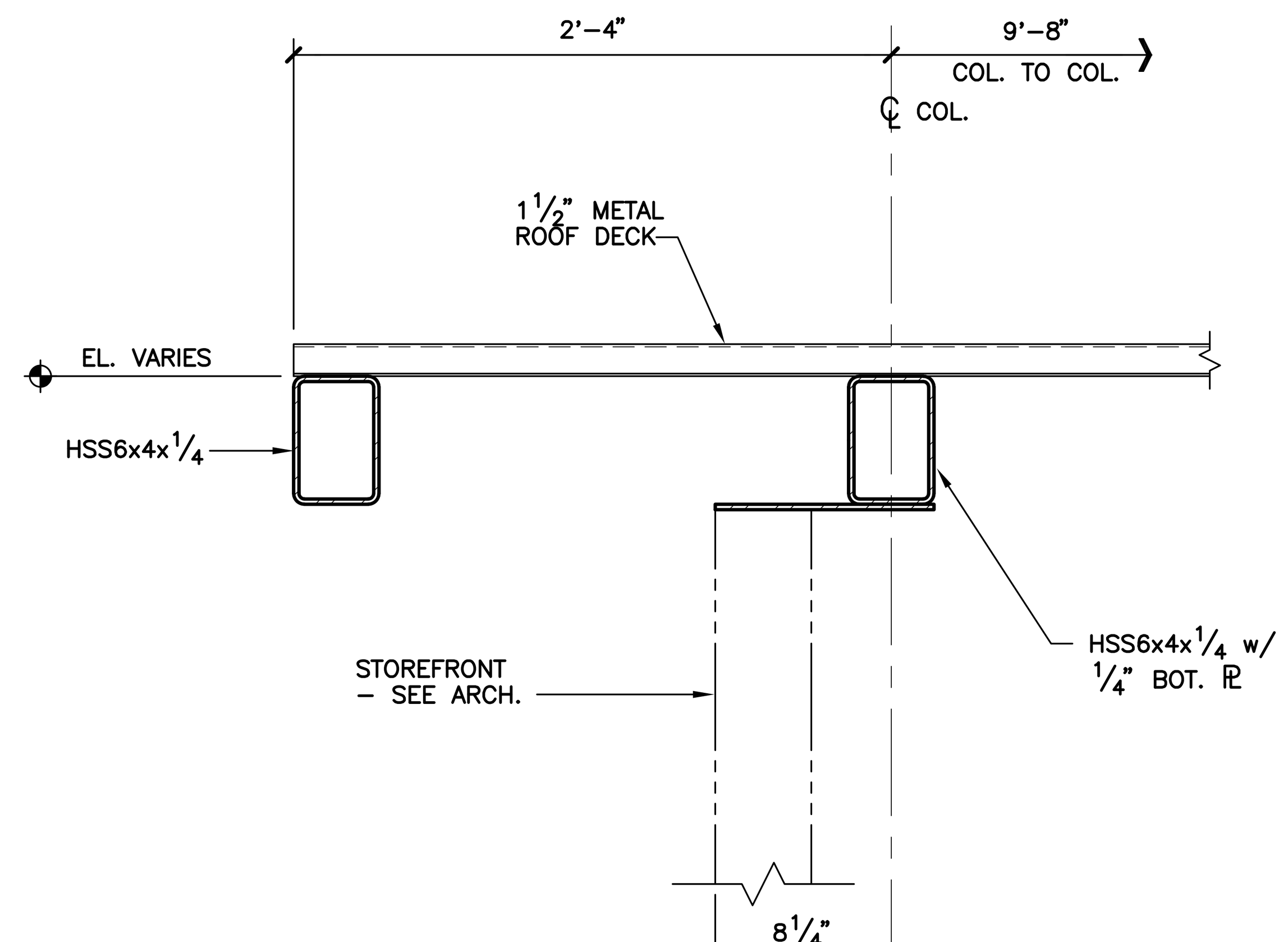
BASE PLATE DETAIL (1 / S303) SBGD520

SCALE: 1 1/2" = 1'–0



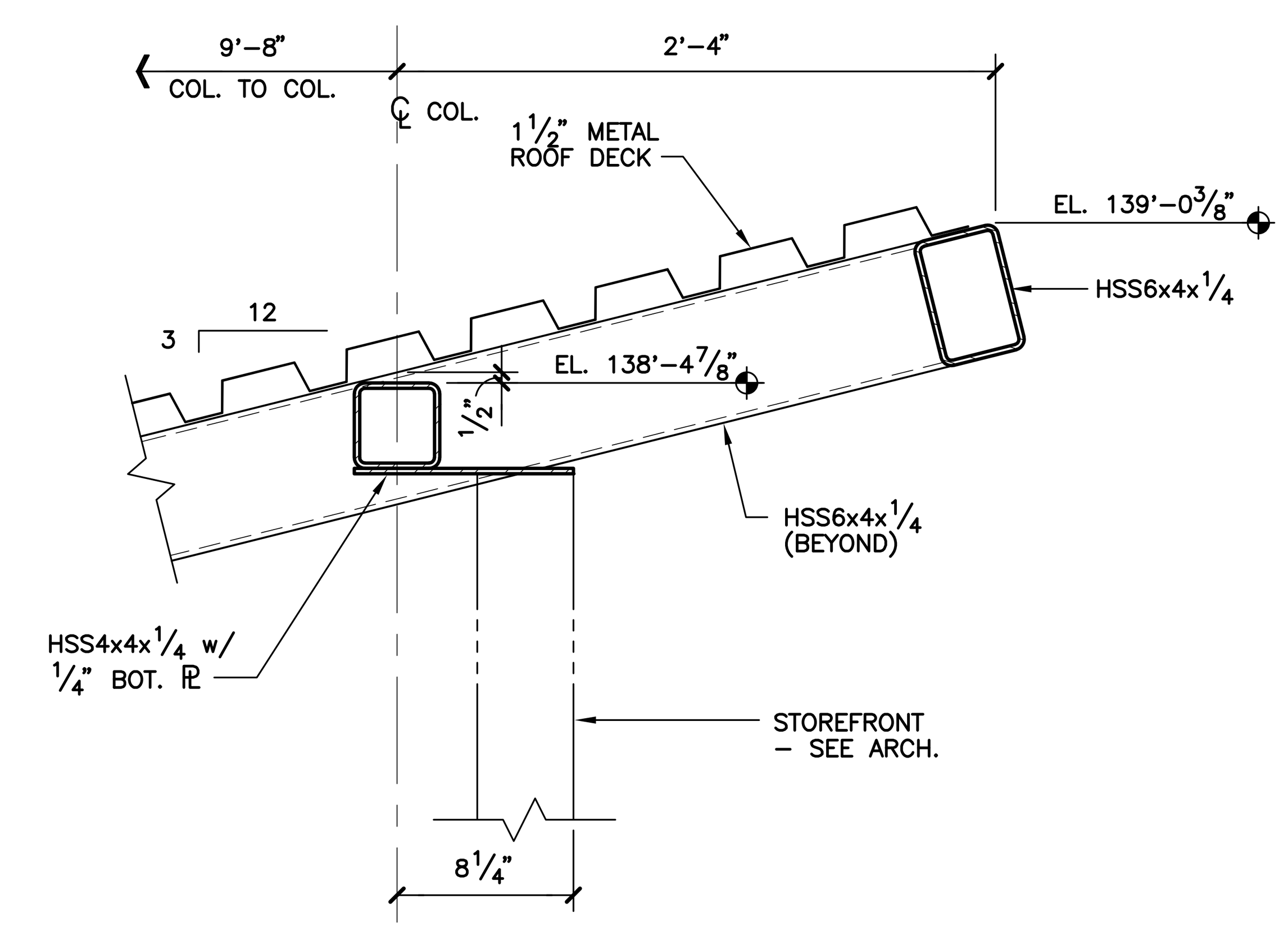
SECTION (F / S303) SRGS451

SCALE: 1 1/2" = 1'–0



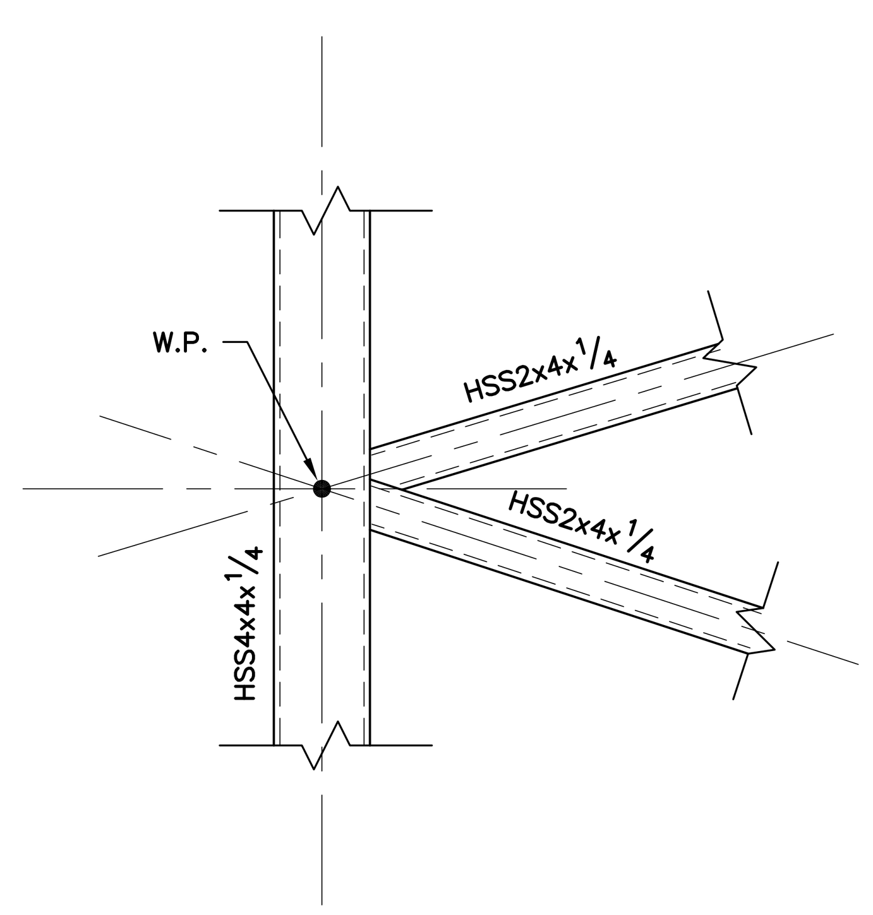
SECTION (G / S303) SRGS451A

SCALE: 1 1/2" = 1'–0



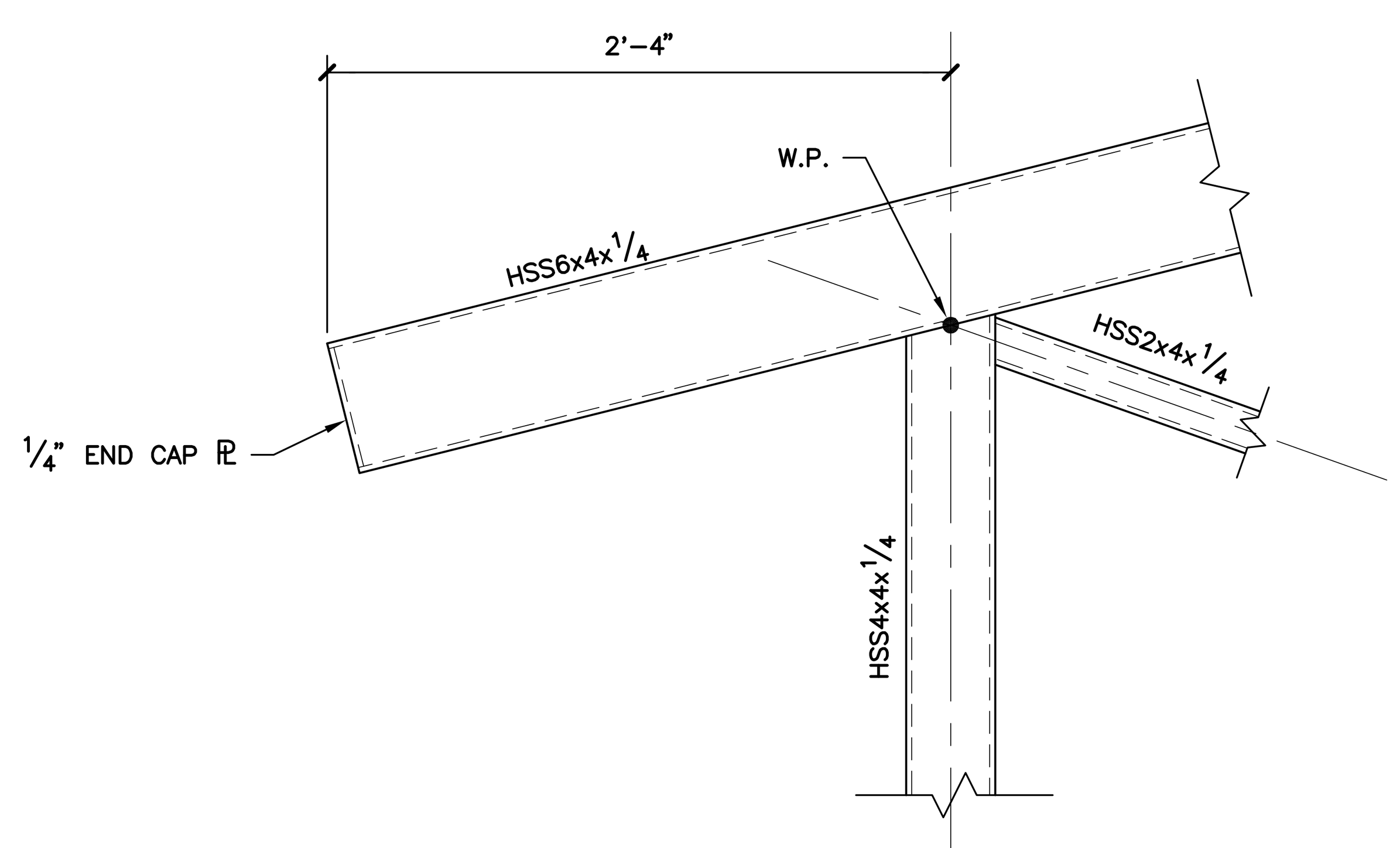
SECTION (H / S303) SRGS451B

SCALE: 1 1/2" = 1'–0



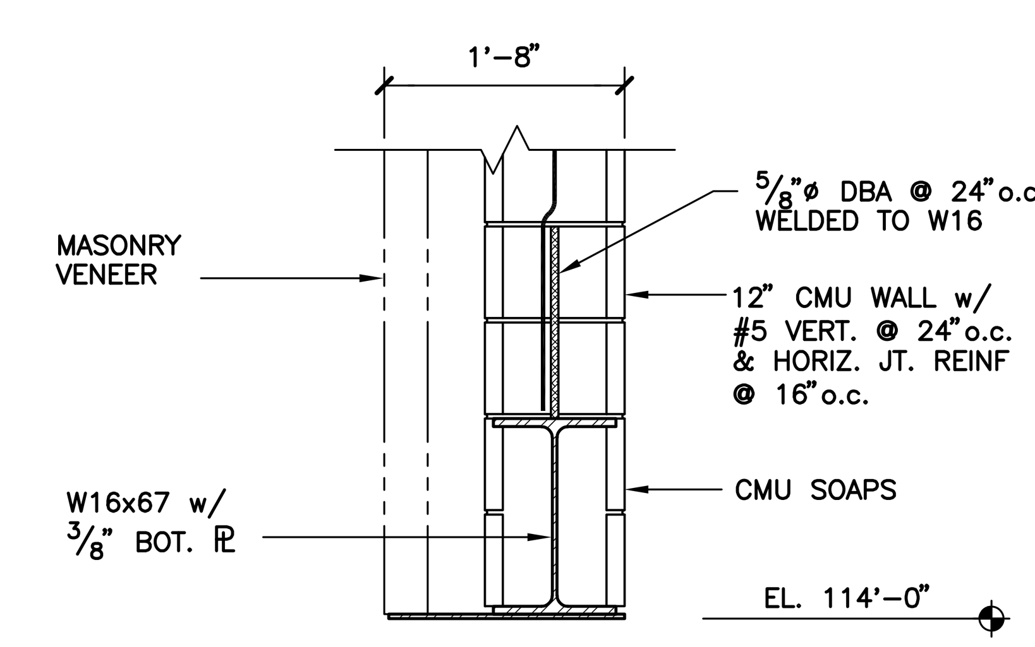
SECTION (J / S303) SRGS451C

SCALE: 1 1/2" = 1'–0



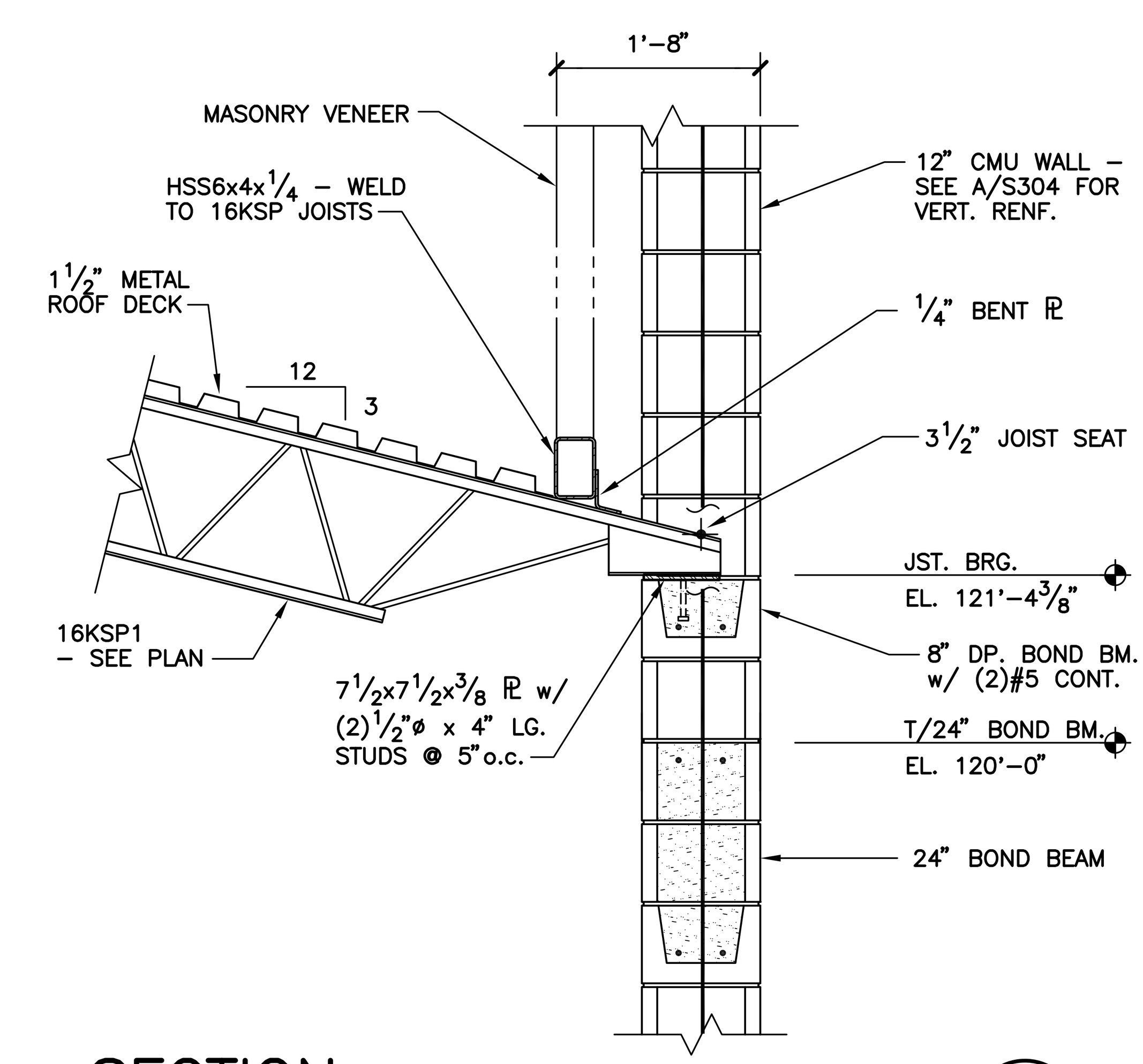
SECTION (K / S303) SRGS451D

SCALE: 1 1/2" = 1'–0



SECTION (L / S303) MNES061

SCALE: 3/4" = 1'–0


1'–8"
MASONRY VENEER
HSS6x4x1/4 – WELD TO 16KSP JOISTS
12" CMU WALL – SEE A/S304 FOR VERT. RENF.
1 1/2" METAL ROOF DECK
1/4" BENT ℙ
12
3
3 1/2" JOIST SEAT
JST. BRG.
EL. 121'–4 3/8"
16KSP1 – SEE PLAN
8" DP. BOND BM. w/ (2)#5 CONT.
7 1/2x7 1/2x3/8 ℙ w/ (2)1/2"ø x 4" LG. STUDS @ 5"o.c.
T/24" BOND BM.
EL. 120'–0"
24" BOND BEAM

SECTION (M / S303) MRES350Z

SCALE: 3/4" = 1'–0

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Project No.	1639.02
Date	11/28/17

Revisions

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19144
STATE OF
INDIANA
PROFESSIONAL ENGINEER

Clarksville Fire Department
Station No. 1

Stansifer Avenue
Clarksville, IN 47129

Town of Clarksville

New Facility

Sheet

S-303

SLESSER ENGINEERING, INC.
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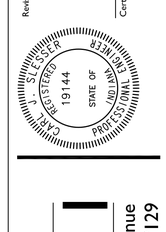

Clarksville Fire Department Station No. 1

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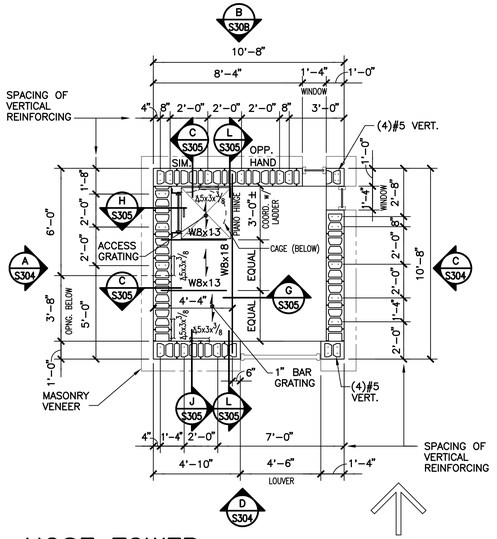
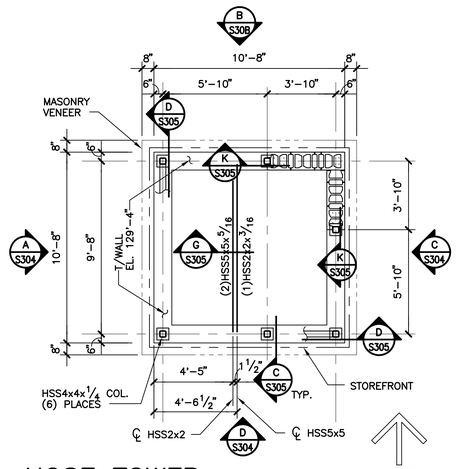
S-304



HOSE TOWER — HOSE PLATFORM PLAN

SRBP094E — 1 S304

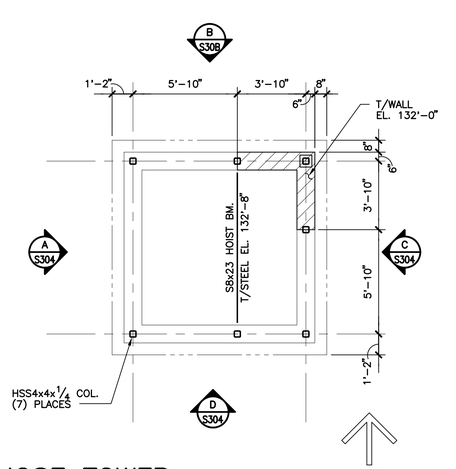
SCALE: 1/4" = 1'-0



HOSE TOWER — COLUMN BEARING PLAN

SRBP094D — 2 S304

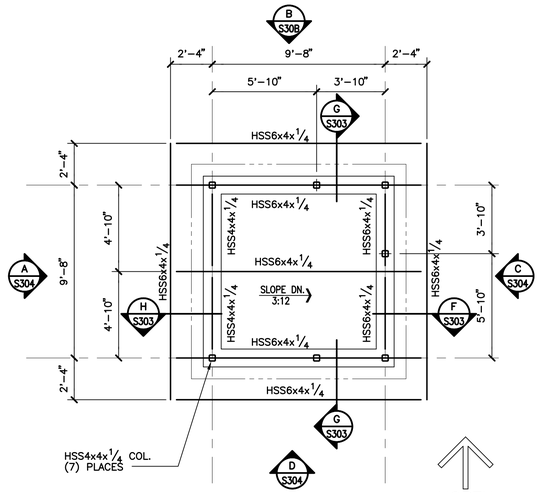
SCALE: 1/4" = 1'-0



HOSE TOWER — HOIST BEAM PLAN

SRBP094C — 3 S304

SCALE: 1/4" = 1'-0



HOSE TOWER ROOF FRAMING PLAN

SRBP094 — 4 S304

SCALE: 1/4" = 1'-0

WEST ELEVATION

SRBE069A — A S304

SCALE: 1/4" = 1'-0

NORTH ELEVATION

SRBE069 — B S304

SCALE: 1/4" = 1'-0

EAST ELEVATION

SRBE069C — C S304

SCALE: 1/4" = 1'-0

SOUTH ELEVATION

SRBE069B — D S304

SCALE: 1/4" = 1'-0

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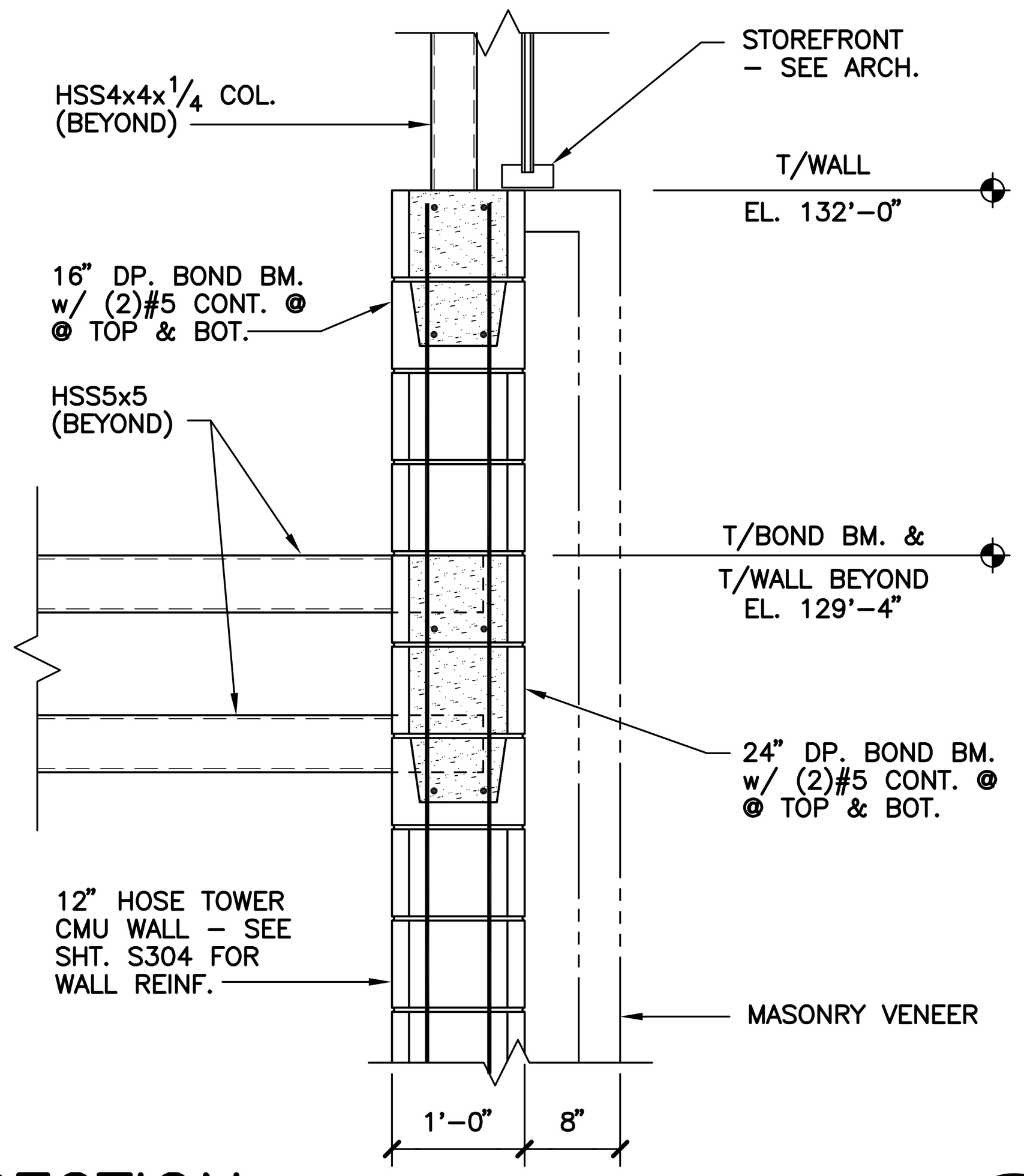
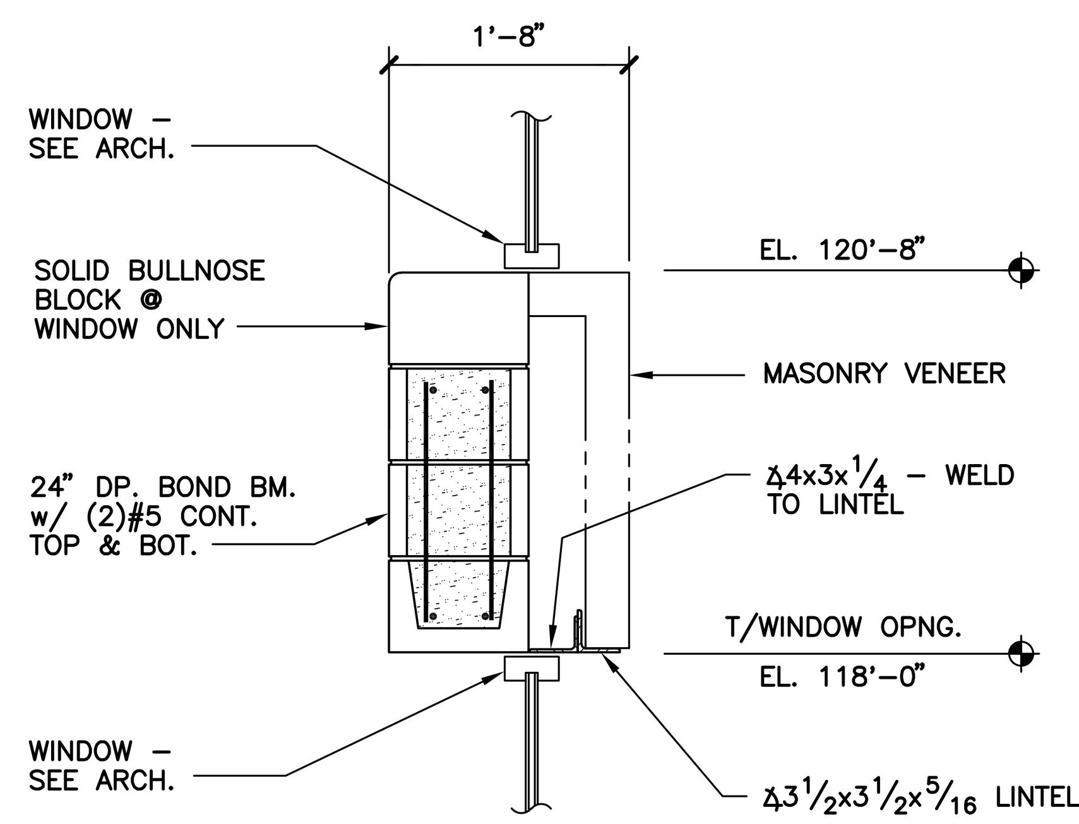


SECTION MRES350S A S305
SCALE: 3/4" = 1'–0



SECTION MRES350U B S305
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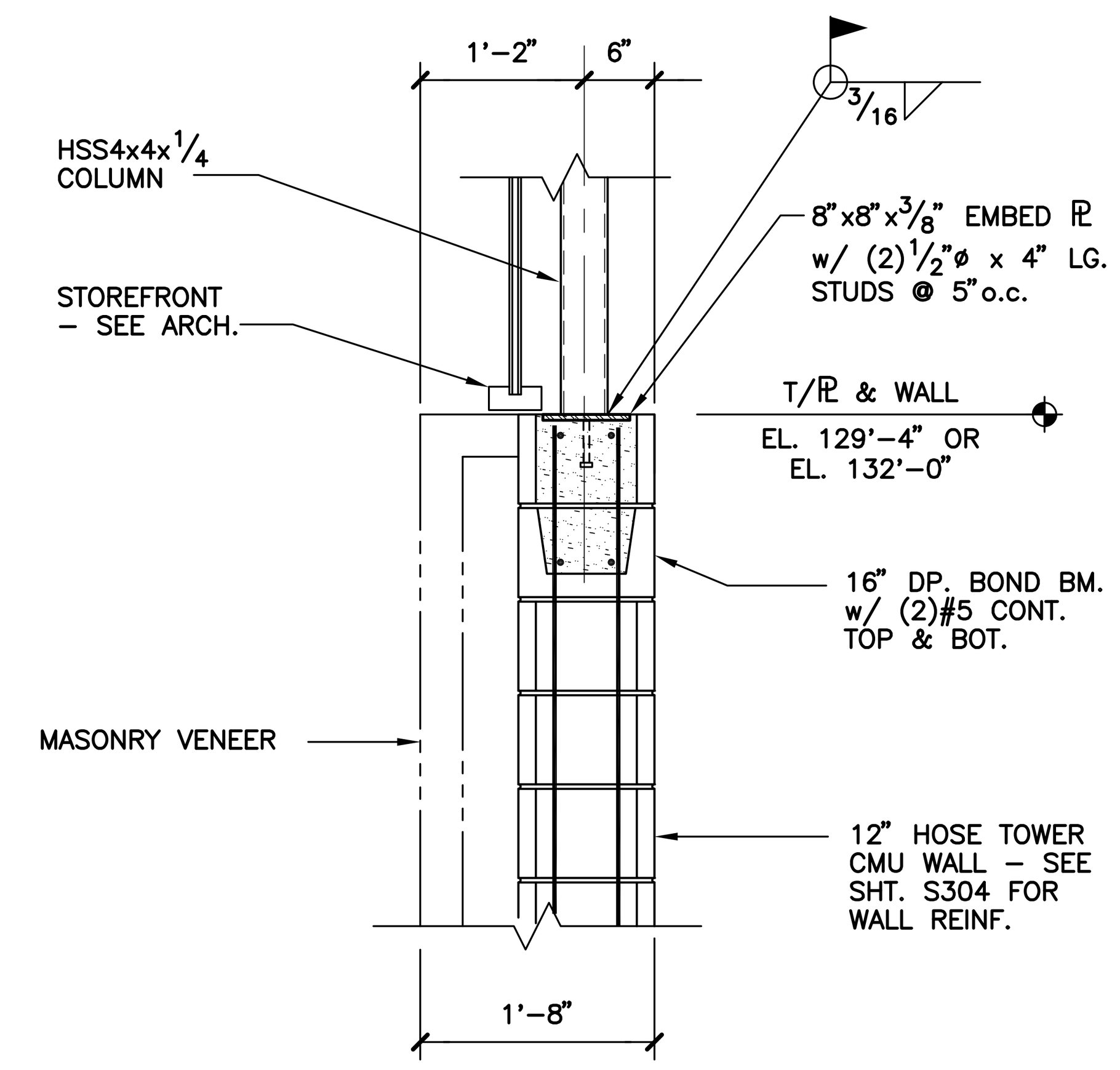
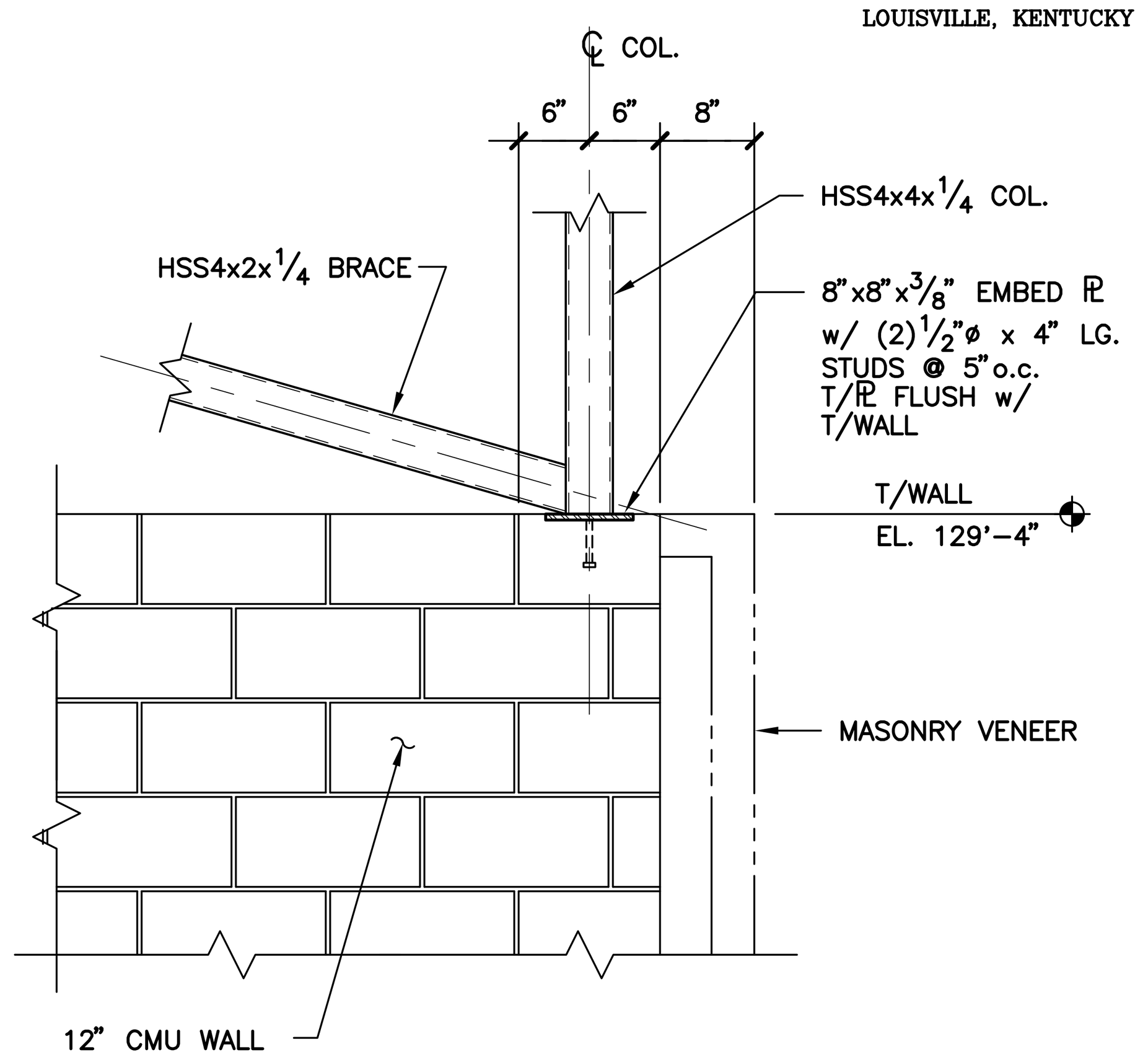


SECTION MRES350W C S305
SCALE: 3/4" = 1'–0



SECTION MRES350X D S305
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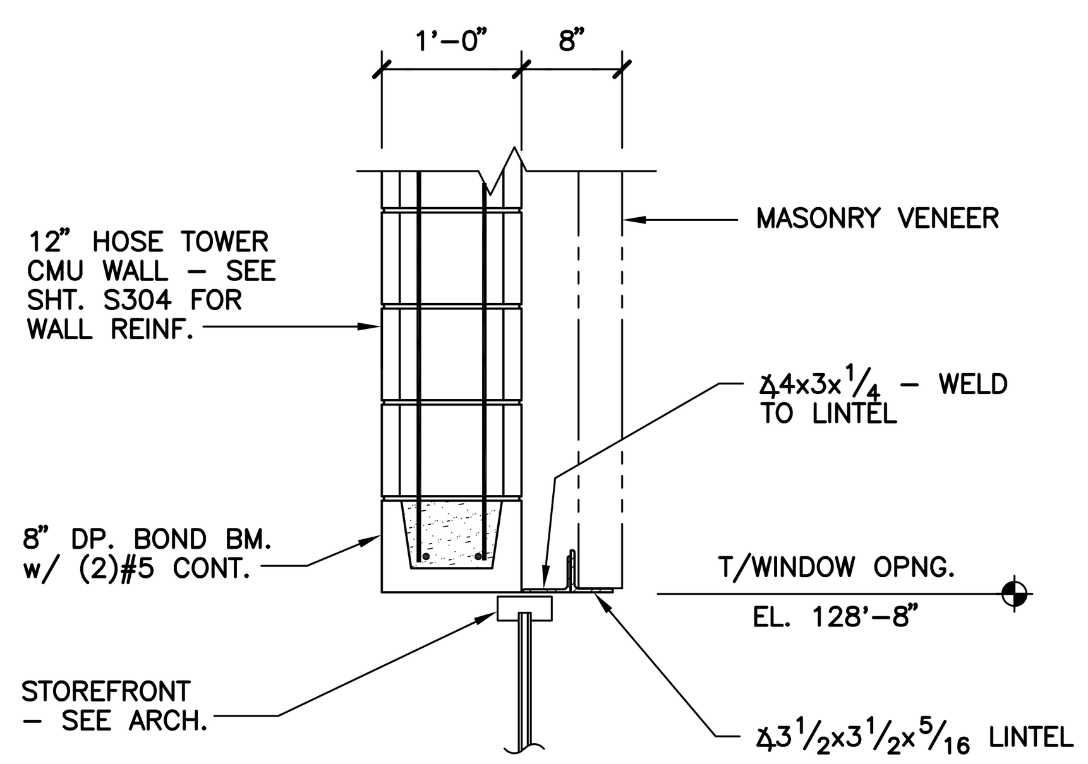
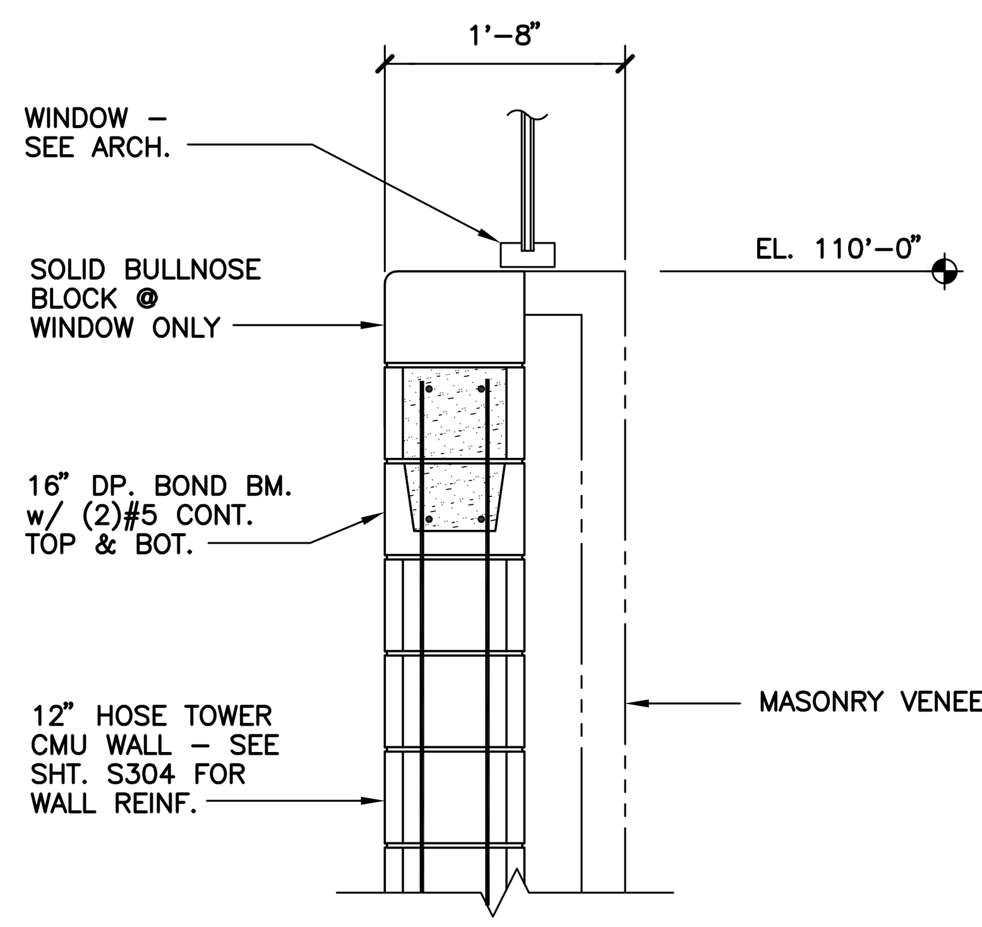
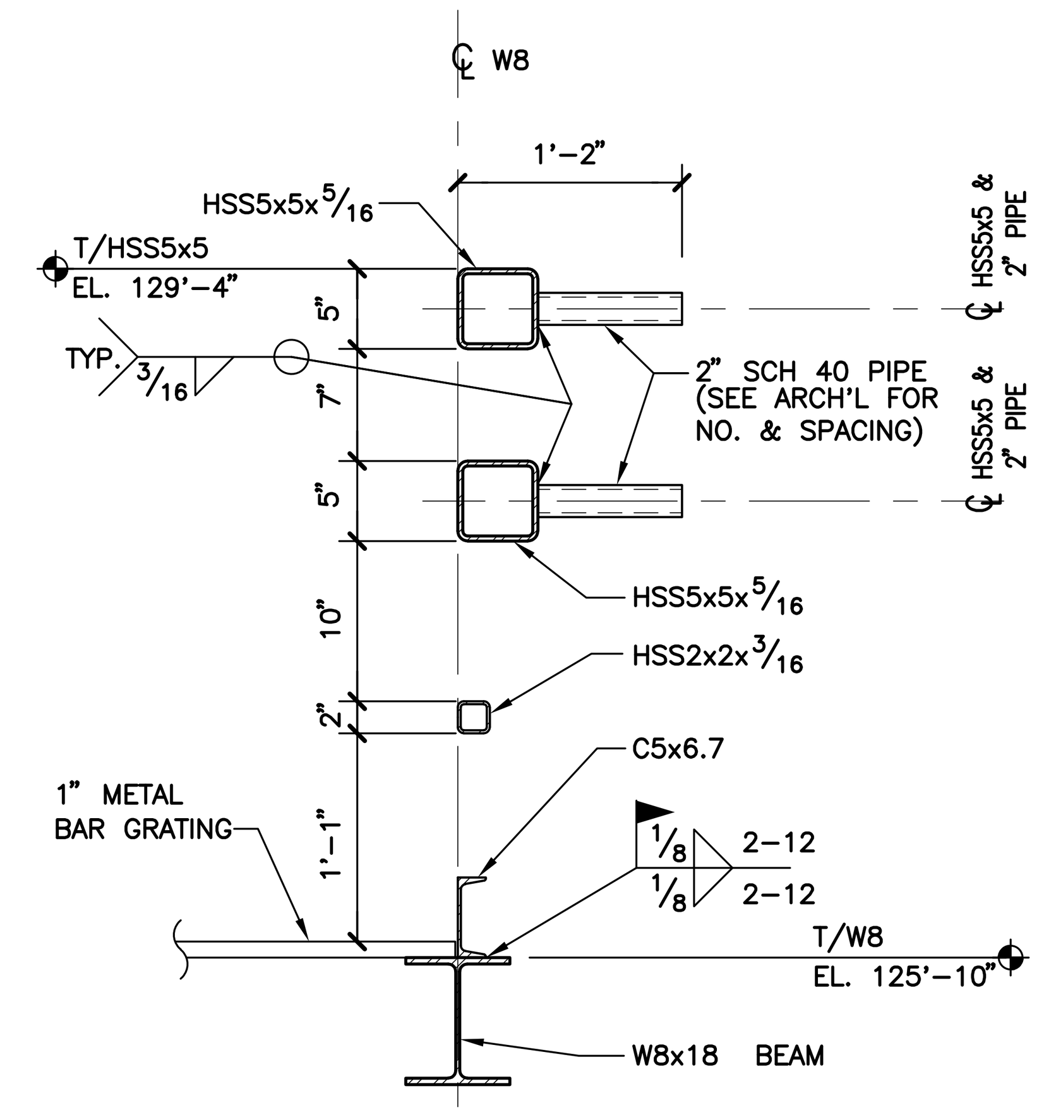


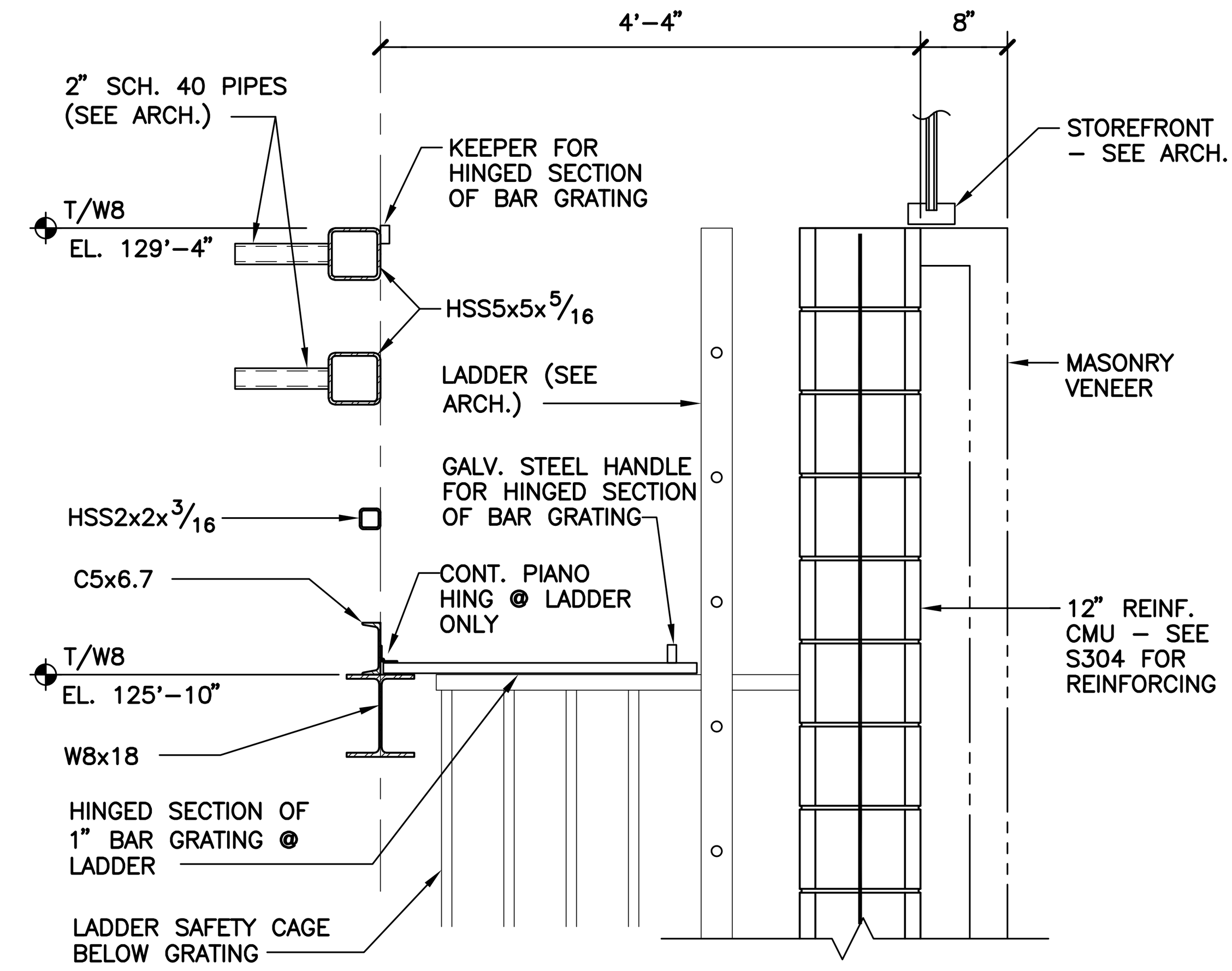
SECTION MRES350T E S305
SCALE: 3/4" = 1'–0



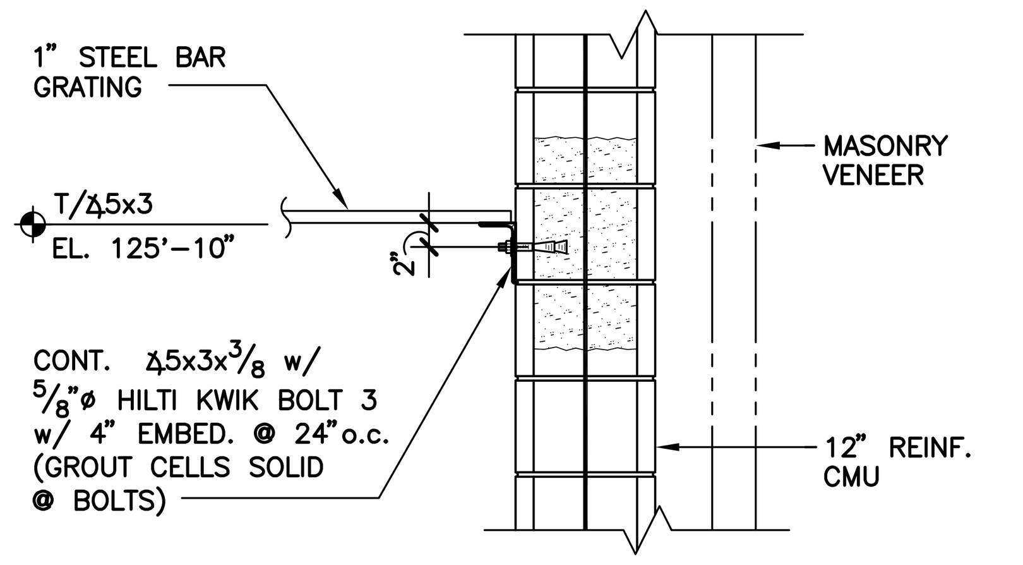
SECTION MRES350V F S305
SCALE: 3/4" = 1'–0



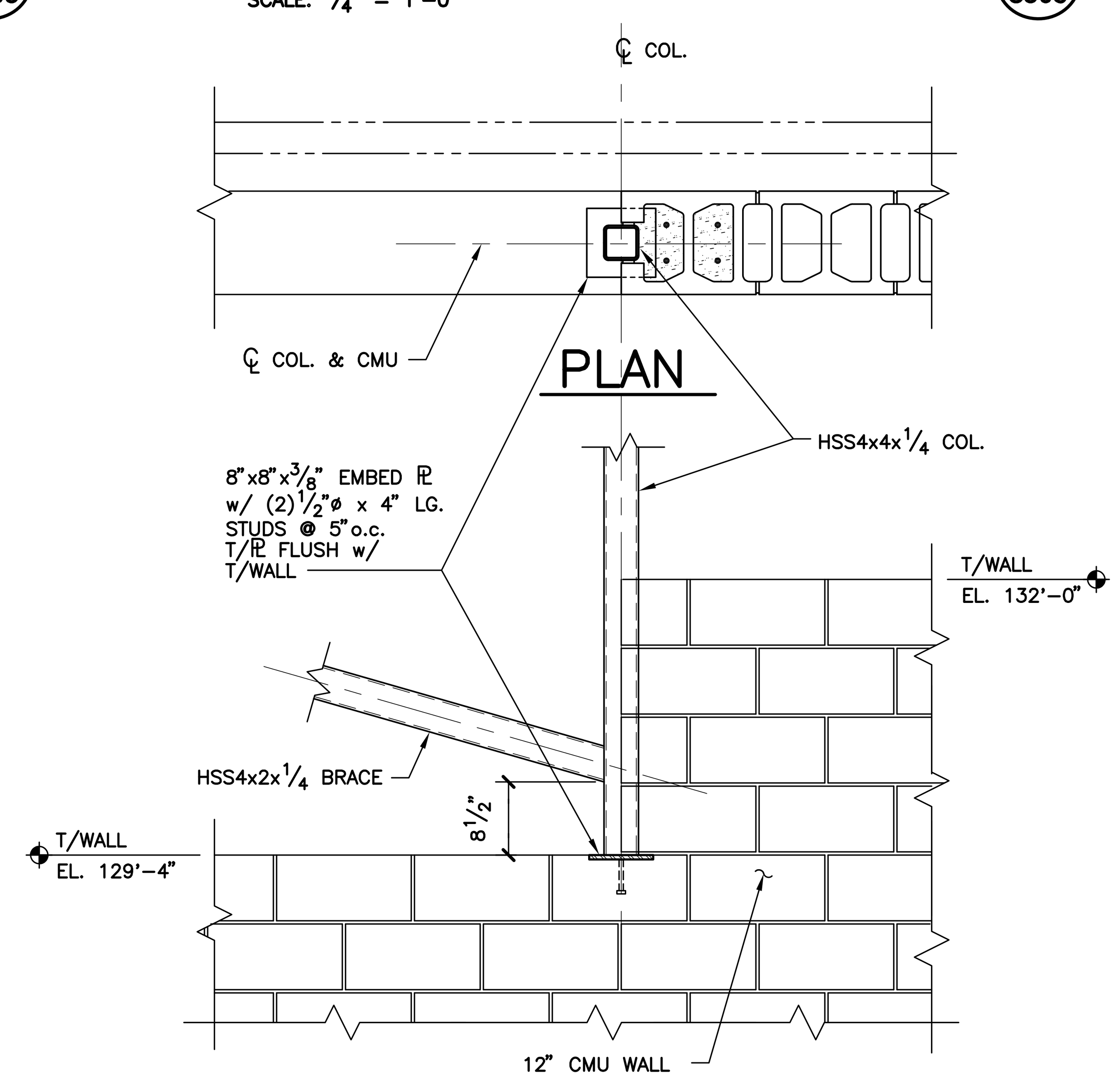
SECTION SOFS222 G S305
SCALE: 1" = 1'–0


4'–4"
8"
2" SCH. 40 PIPES (SEE ARCH.)
KEEPER FOR HINGED SECTION OF BAR GRATING
STOREFRONT – SEE ARCH.
T/W8
EL. 129'–4"
HSS5x5x5/16
LADDER (SEE ARCH.)
MASONRY VENEER
GALV. STEEL HANDLE FOR HINGED SECTION OF BAR GRATING
HSS2x2x3/16
C5x6.7
CONT. PIANO HING @ LADDER ONLY
12" REINF. CMU – SEE S304 FOR REINFORCING
T/W8
EL. 125'–10"
W8x18
HINGED SECTION OF 1" BAR GRATING @ LADDER
LADDER SAFETY CAGE BELOW GRATING

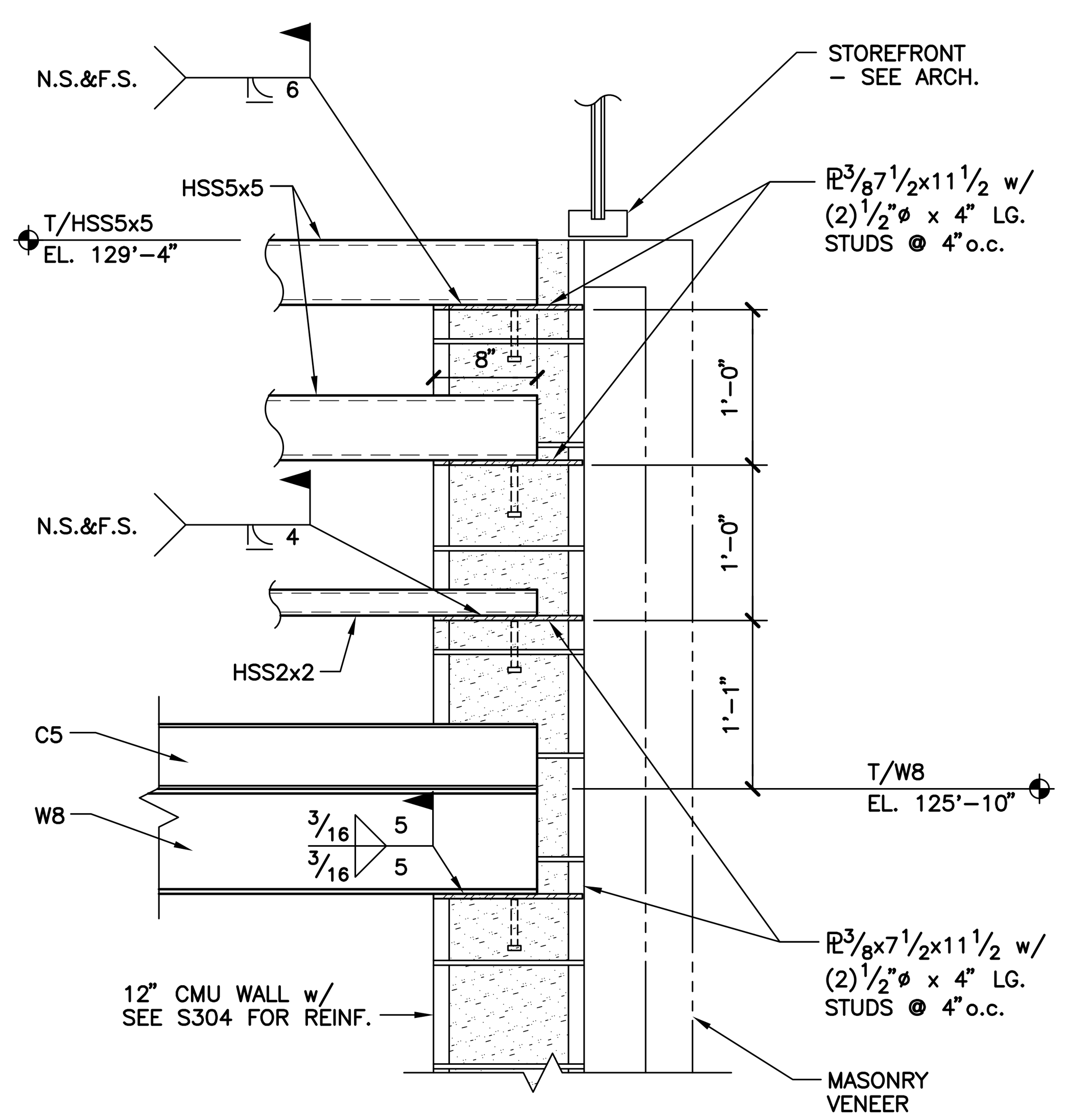
SECTION SOES140 H S305
SCALE: 3/4" = 1'–0



SECTION SOES140A J S305
SCALE: 3/4" = 1'–0



SECTION MRES350Y K S305
SCALE: 3/4" = 1'–0



SECTION SOFS222A L S305
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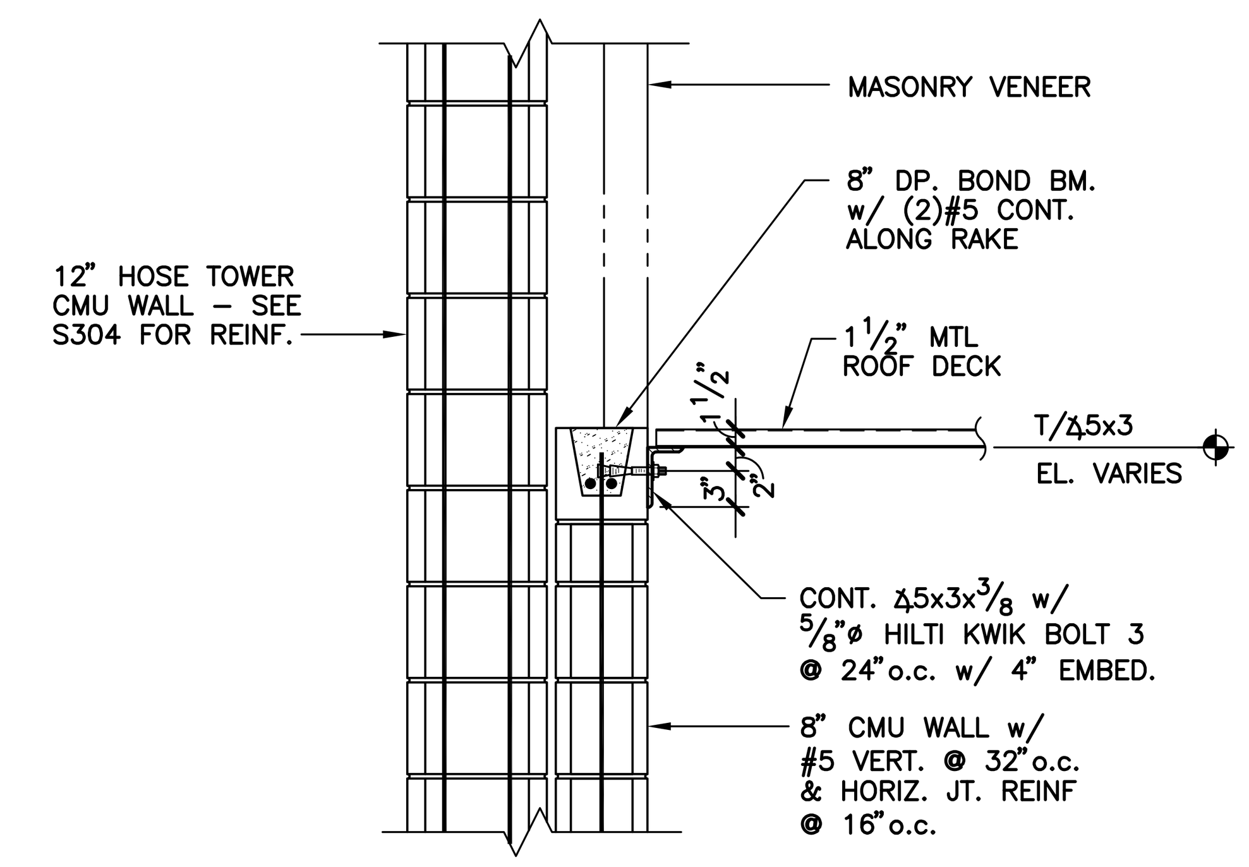

SECTION MRES350M M S305
SCALE: 3/4" = 1'–0

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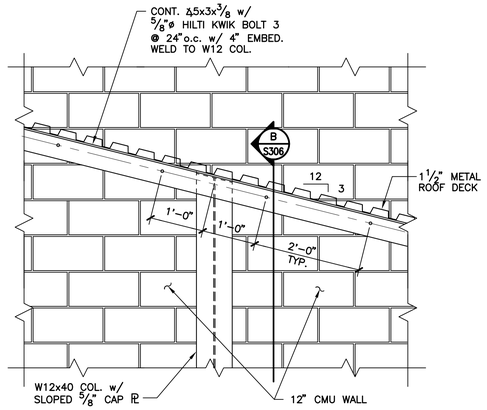
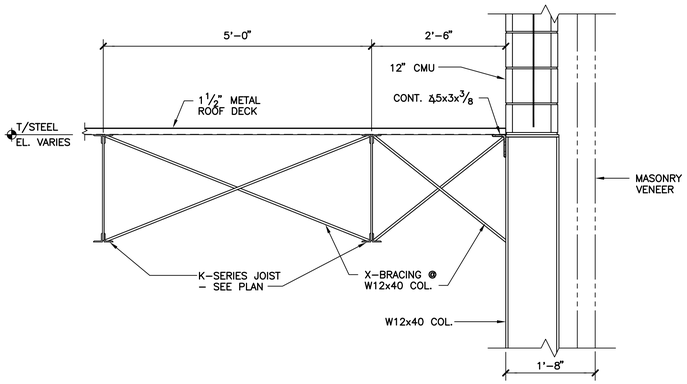


SECTION
SCALE: 3/4" = 1'-0
MRES350Q A S306



SECTION
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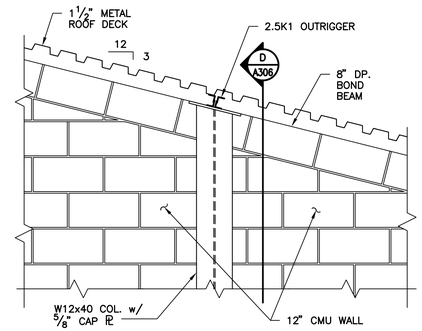
MRES350R B S306



SECTION
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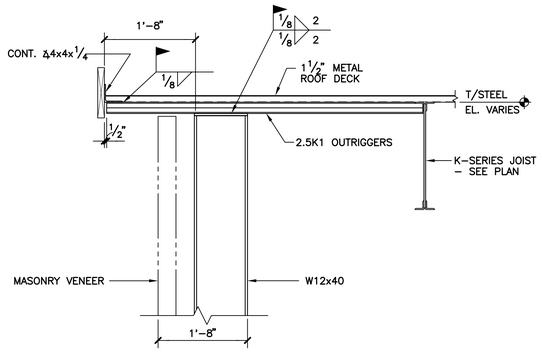
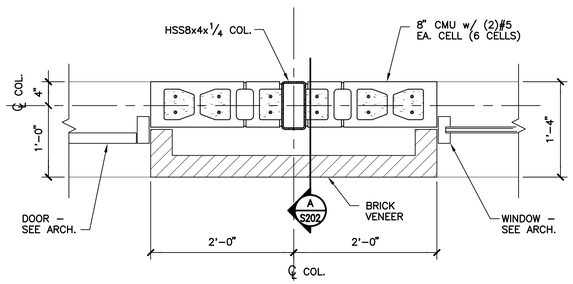
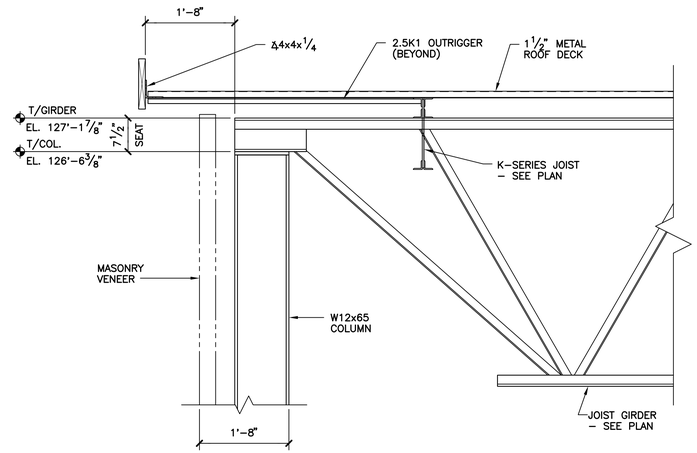
MRES350P C S306



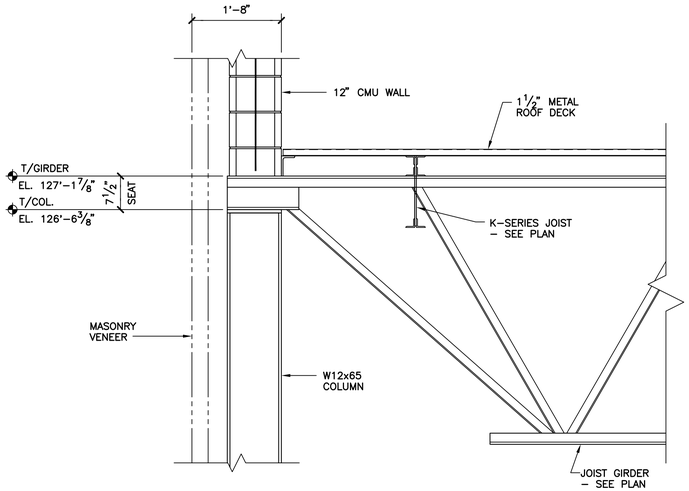
SECTION
SCALE: 3/4" = 1'-0
MRES350N D S306



PLAN DETIAL
SCALE: 1" = 1'-0
MOFD029 1 S306



SECTION
SCALE: 3/4" = 1'-0
SRES1113D E S306



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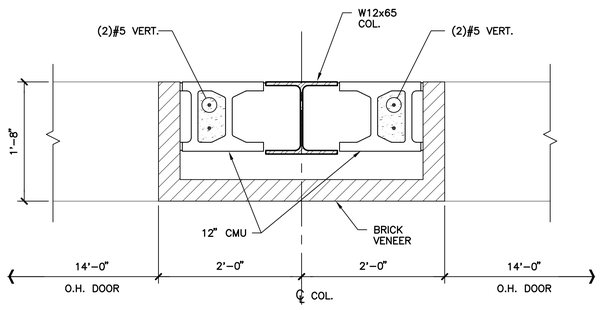
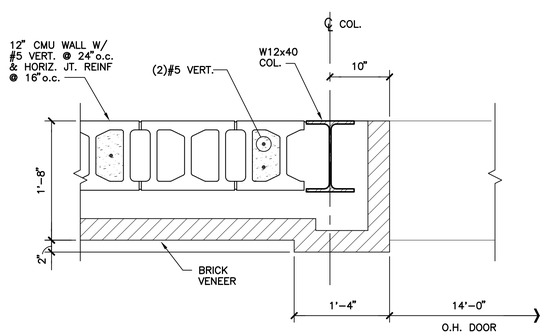
SRES1113E F S306



PLAN DETIAL
SCALE: 1" = 1'-0
MOFD029A 2 S306



PLAN DETIAL
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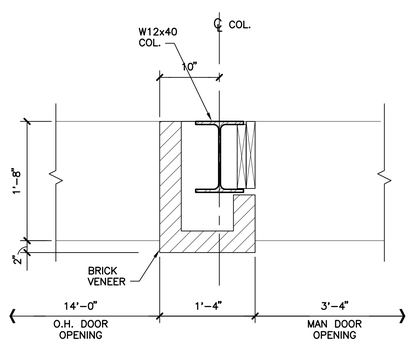
MOFD029B 3 S306



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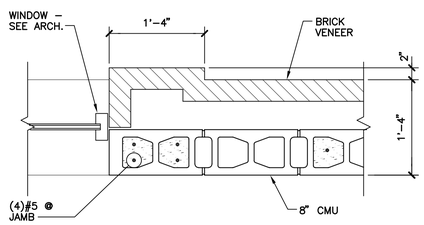

PLAN DETIAL
SCALE: 1" = 1'-0
MOFD029D 5 S306

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